







COVER SHEET

SINGLE FAMILY HOUSE REMODEL AARON CELIOUS 3612 6TH AVENUE, LOS ANGELES, CA 90018

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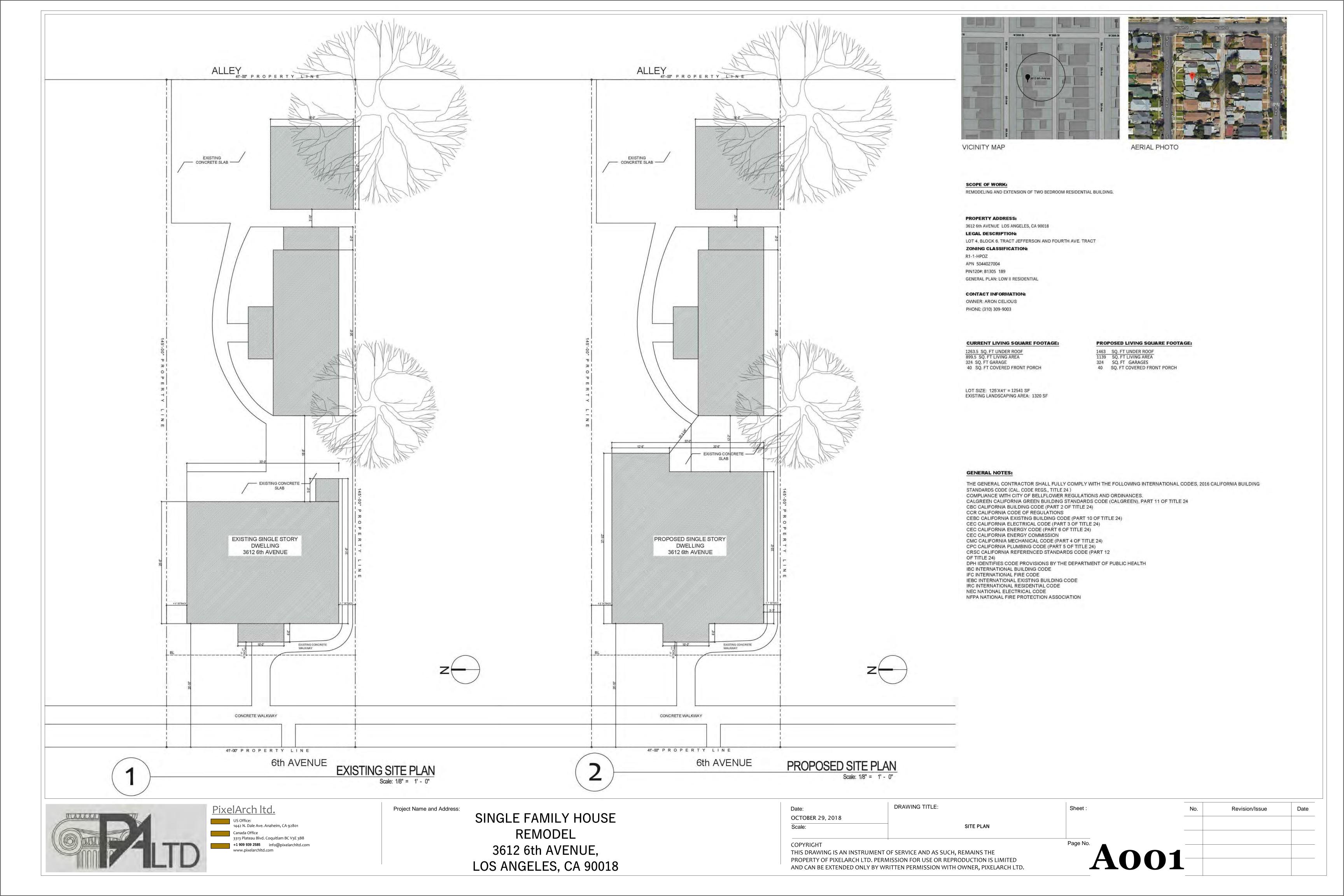
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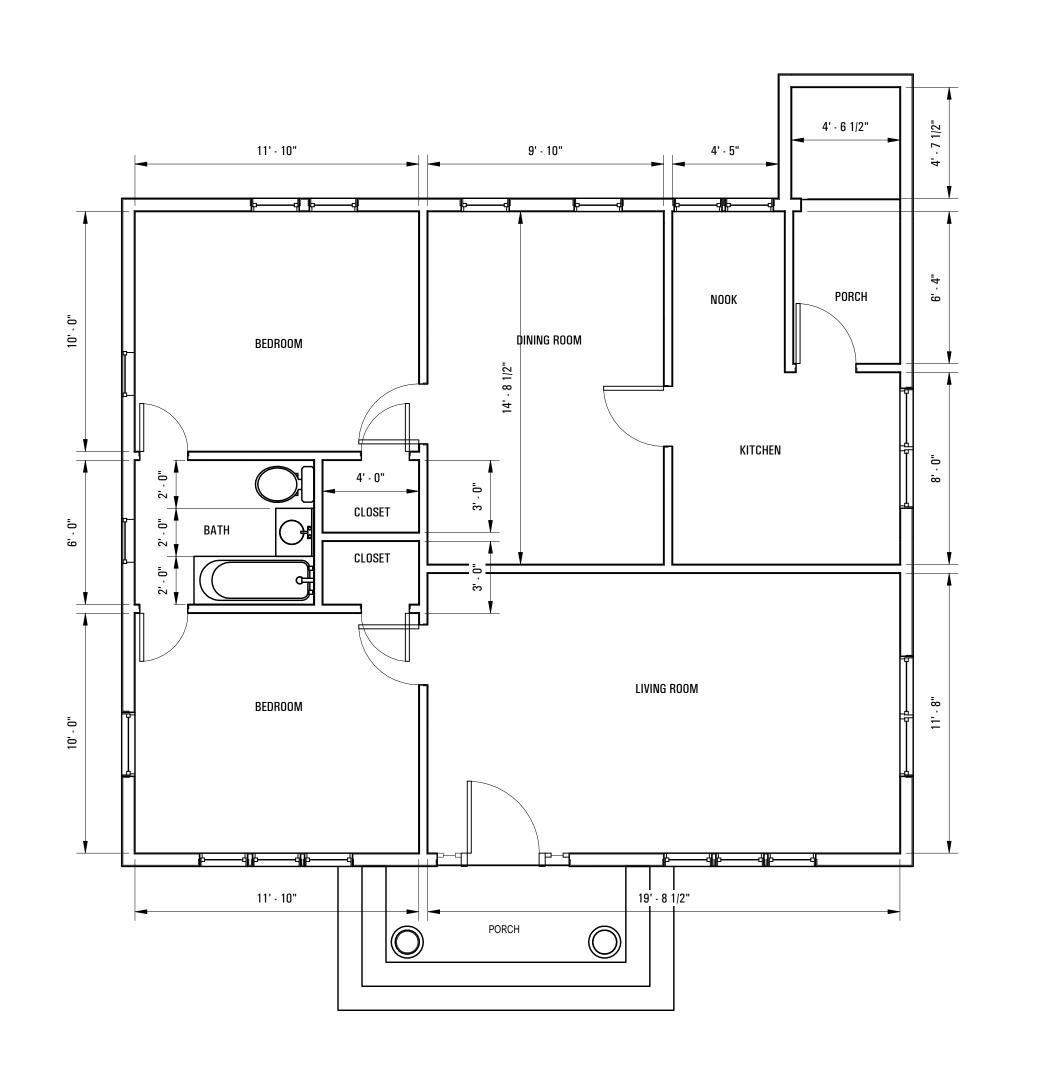
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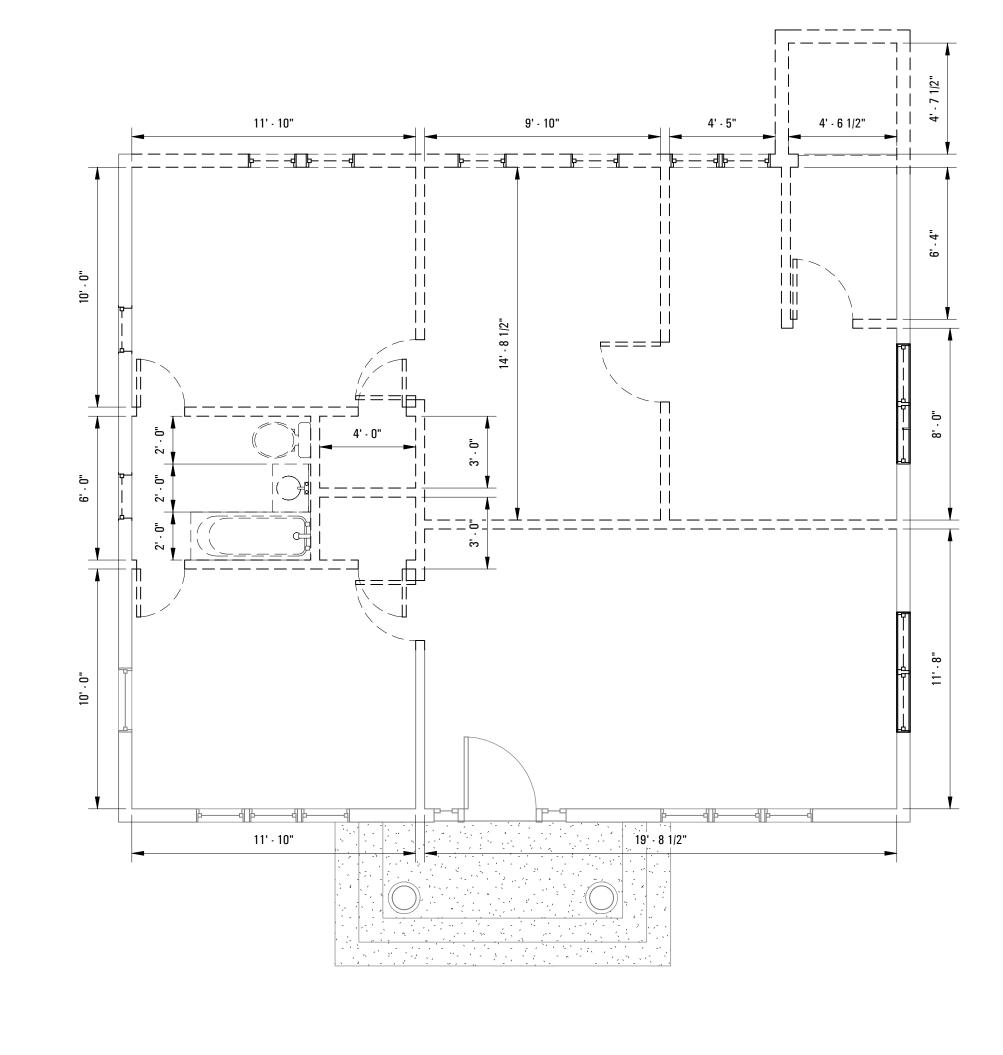
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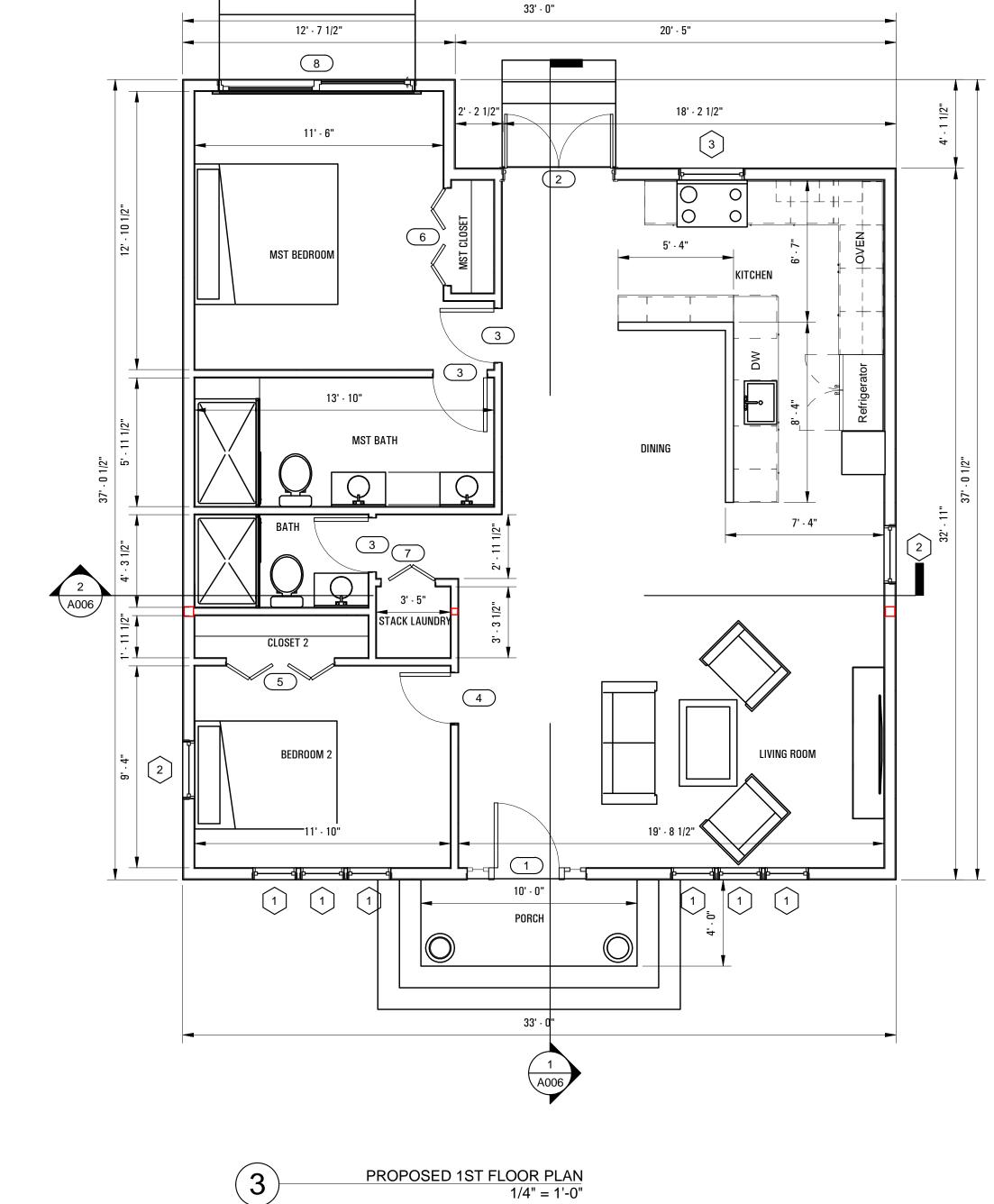
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EX. 1ST FLOOR PLAN 1/4" = 1'-0"

WINDOW TAG

PLAN SYMBOLS

1 DOOR TAG 1 WALL TAG

EXISTING WALLS ☐☐☐☐ EXISTING WALLS
TO BE DEMOLISHED

NEW WALLS

(2)	DEMO 1ST FLOOR PLAN
(Z)	1/4" = 1'-0"

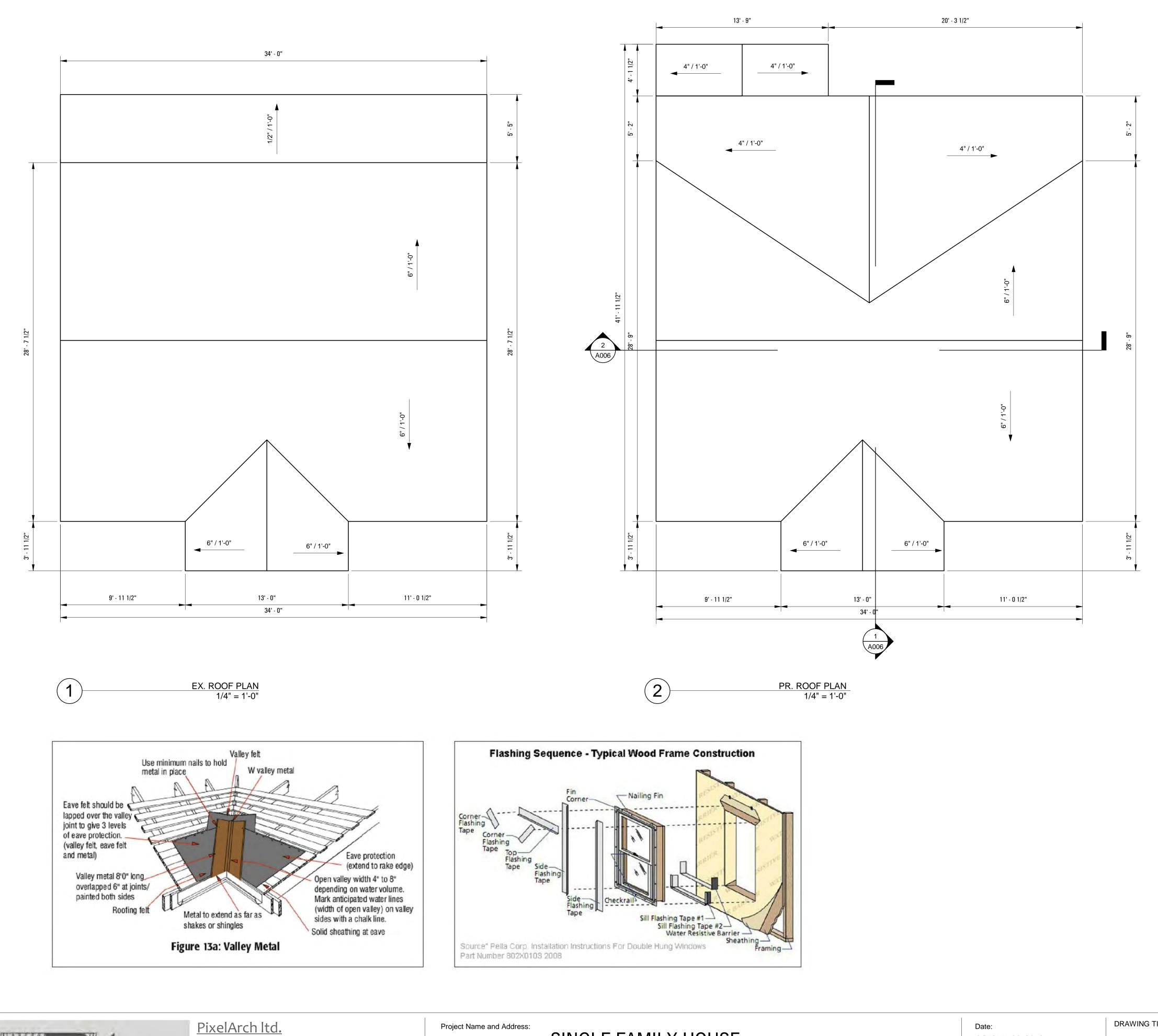
	ROOM SCHEDULE				
Number	Name	Area			
1	PORCH	36 SF			
2	LIVING ROOM	333 SF			
3	DINING	90 SF			
4	KITCHEN	178 SF			
5	MST BEDROOM	155 SF			
6	MST CLOSET	10 SF			
7	MST BATH	82 SF			
9	BEDROOM 2	110 SF			
10	CLOSET 2	16 SF			
11	STACK LAUNDRY	11 SF			
12	BATH	34 SF			
Grand total	,	1058 SF			





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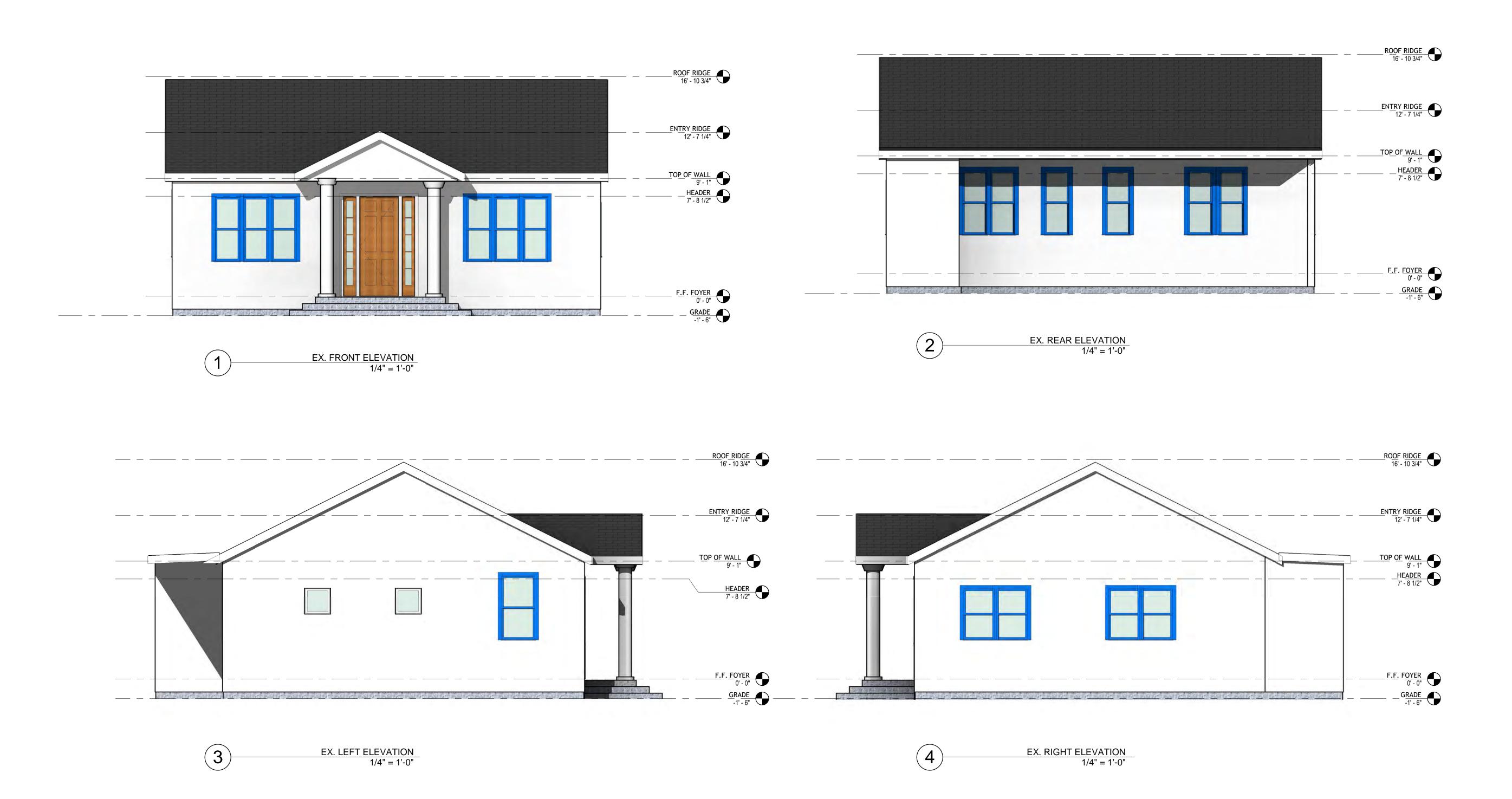
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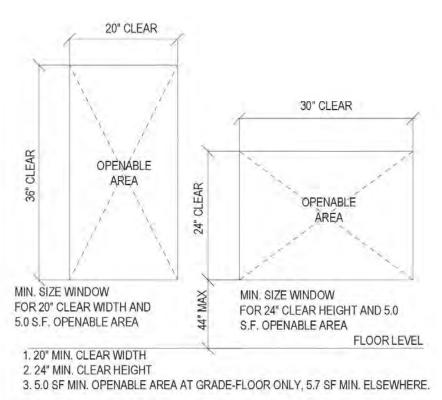
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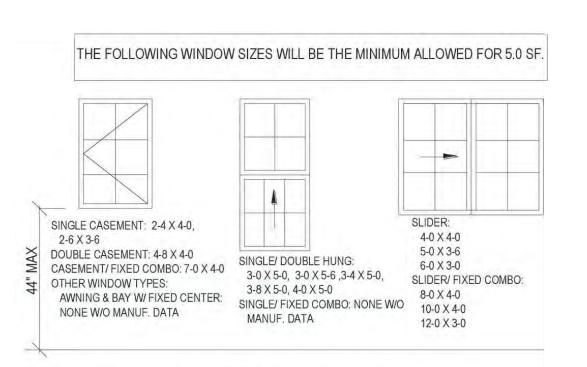




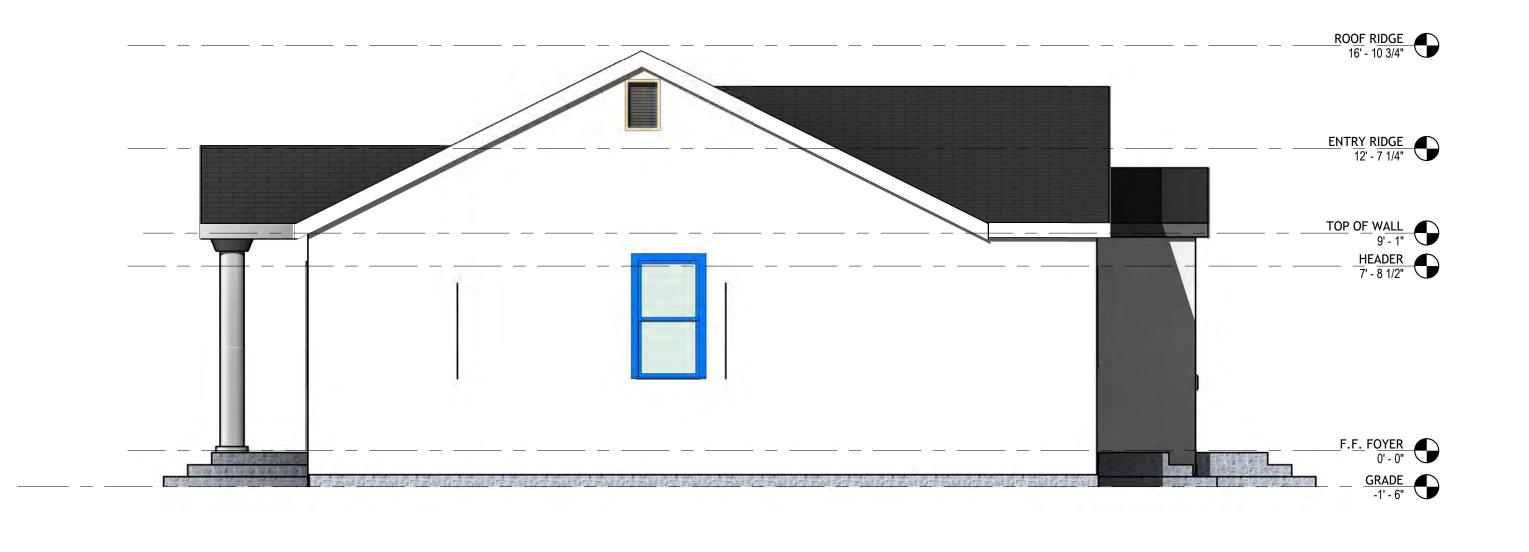
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EMERGENCY ESCAPE/ RESCUE OPENING (R310)

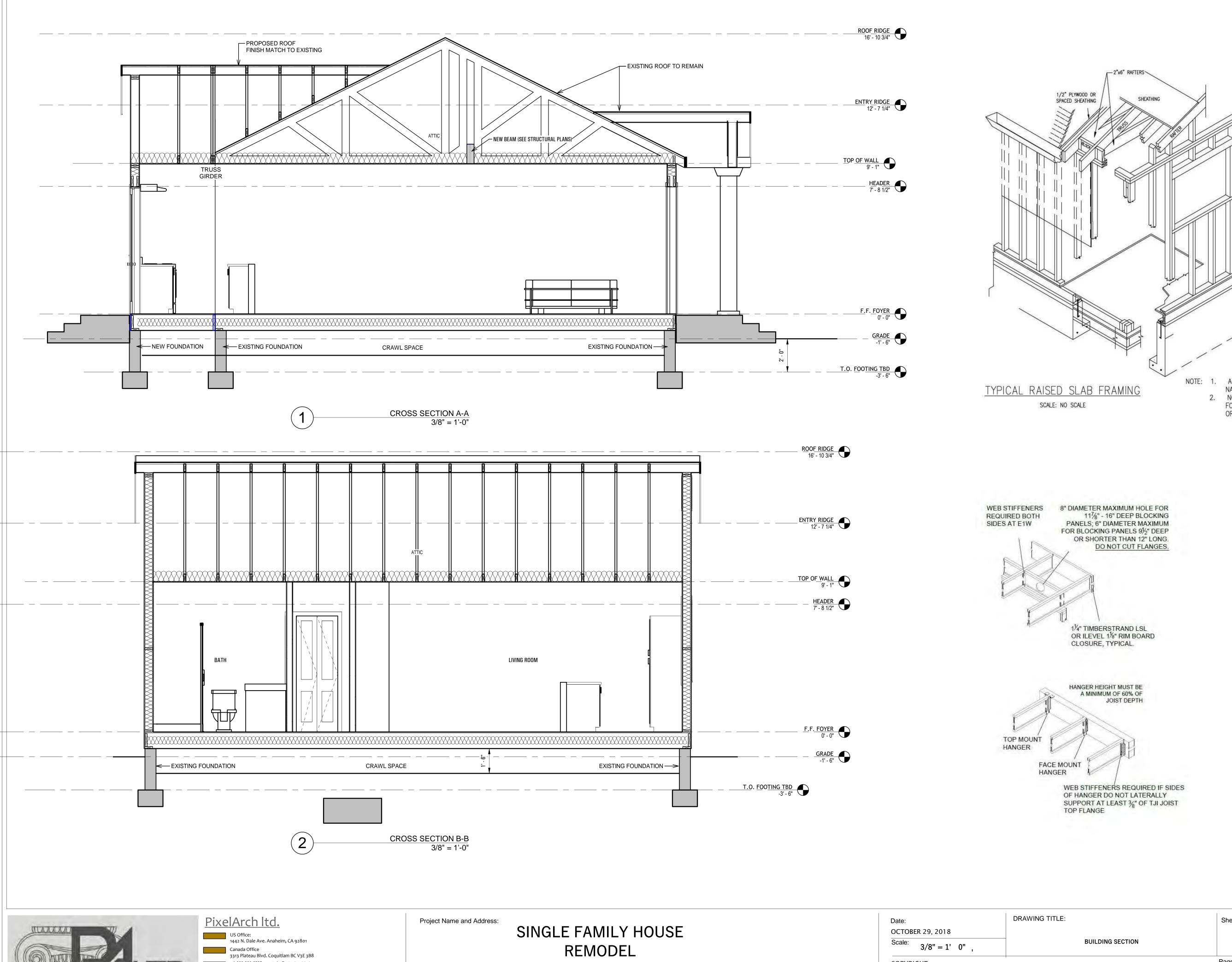


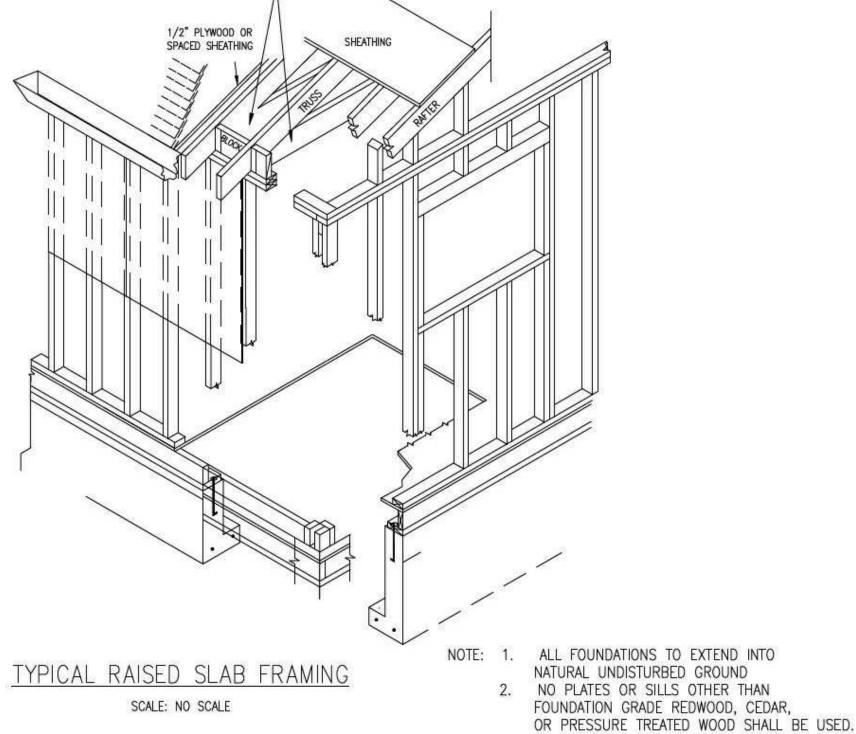
NOTE: SIZES ARE TAKEN FROM DATA SUPPLIED BY WINDOW MANUFACTURERS. HOWEVER, THESE ARE GENERAL DIMENSIONS AND MUST BE VERIFIED WITH ACTUAL WINDOWS INSTALLED TO MEET MINIMUM EGRESS REQUIREMENTS.



PR. RIGHT ELEVATION 1/4" = 1'-0"

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FLOOR FRAMING & TRUSS NOTES

FLOOR: 40 PSF LL. *10 PSF TOP CHORD DL. FLOORS 3 AND 2 SHALL HAVE 22" FLOOR TRUSSES; BOTTOM FLOOR IS CONCRETE SLAB. SUB-FLOOR SHEATHING SHALL BE H 1.125". SHEATHING IS REQUIRED FOR ANY LONGITUDINAL(DRAG) FORCES. TRUSSES SPACED AT 24.0" O/C. TRUSS DRAWING IS FOR ILLUSTRATION ONLE. ALL TRUSSES SHALL BE INSTALLED & BRACED TO MANUFACTURERS DRAWINGS & SPECIFICATIONS.

ALL PLATES ARE 1.5 X 4 UNO DEFLECTION MEETS L/480 LIVE AND L/360 TOTAL LOAD. FASTEN RATED SHEATHING TO ONE FACE OF THIS FRAME.

ALL TRUSSES WILL NOT BE FIELD ALTERED WITHOUT PRIOR BUILDING DEPT. APPROVAL OF ENGINEERING CALCULATION. ALL TRUSSES SHALL HAVE DESIGN DETAILS & DRAWINGS ON SITE FOR FRAMING INSECTION. ALL DLOOR TRUSSES SHALL CARRY MANUFACTURERS STAMP.

NAILING NOTES: (PER IRC TABLE R602.3(1))

TOE NAIL (3)8d JOIST TO SILL OR GIRDER **BRIDGING TO JOIST** TOE NAIL EA, END (2)-8d SOLE PLATE TO JOIST OR BLK'G FACE NAIL 16d @ 16"OC STUD TO SOLE PLATE TOE NAIL (4)-8d, END NAIL (2) 16d TOP PLATE TO STUD END NAIL (2)-16d

DOUBLE STUDS FACE NAIL 16d @ 24" OC DOUBLE TOP PLATES FACE NAIL 16d @ 16" OC 16d @ 16" OC ALONG EA. EDGE CONTINUOUS HEADER, TWO PIECES BUILT , UP HEADER, TWO PIECES W/ 1/2" SPACER 16d @ 16" OC ALONG EA, EDGE

TOE NAIL (3)-8d CEILING JOISTS TO PLATE CONTINUOUS HEADER TO STUD TOE NAIL (3)-8d CEILING JOISTS, LAPS OVER PARTITIONS FACE NAIL (3)-10d CEILING JOISTS TO PARALLEL RAFTERS FACE NAIL (3)-10d RAFTER TO PLATE TOE NAIIL (2)-16d 1" BRACE TO EACH STUD AND PLATE FACE NAIL (2)-8D BUILT , UP CORNERS STUDS 10d @ 24" OC 2" PLANKS (2)-16d @ EA.BRG.

TOP PLATES, LAPS AND INTERSECTIONS FACE NAIL (2)-16d

1/2" PLYWOOD SUBFLOOR EDGES 8D @ 6" OC INTERMEDIATE 8d @ 12" OC

2x MULTIPLE JOISTS , STAGGER @ 15" OC W/(2) @ EA, END OR SPLICE

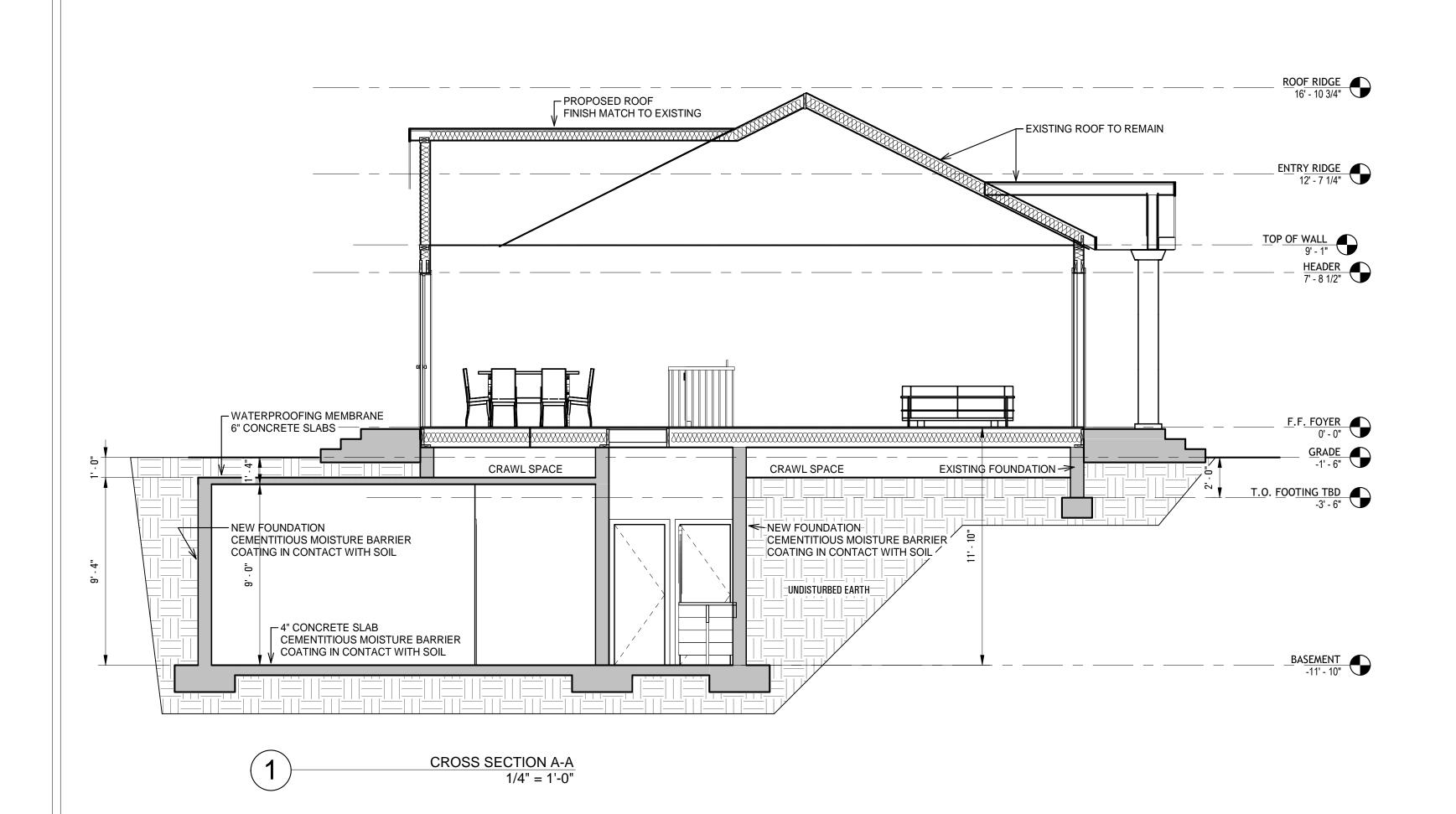
(3) OR FEWER (4) OR MORE

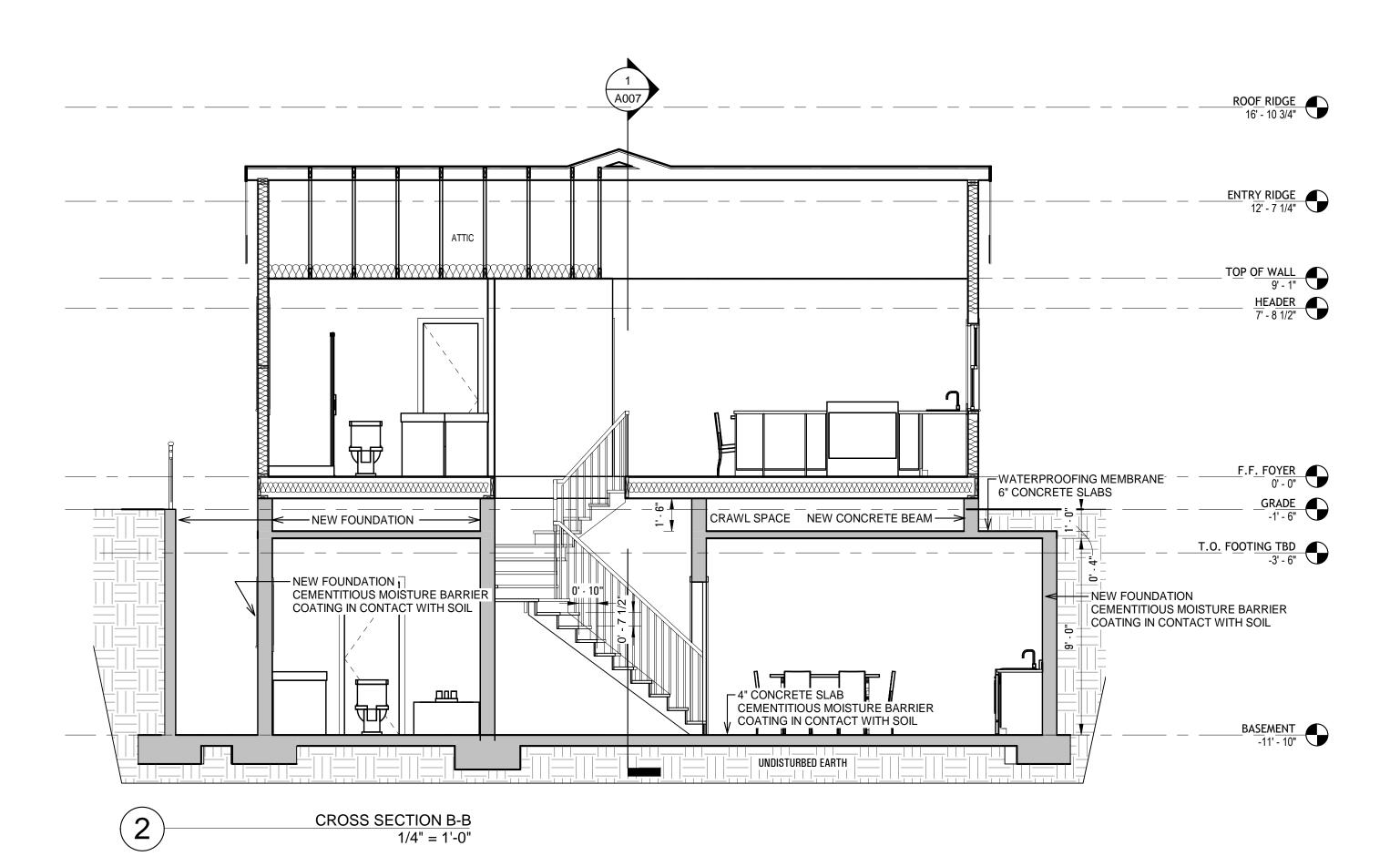
1/2" DIA M.B. W/ STANDARD NUT AND WASHERS

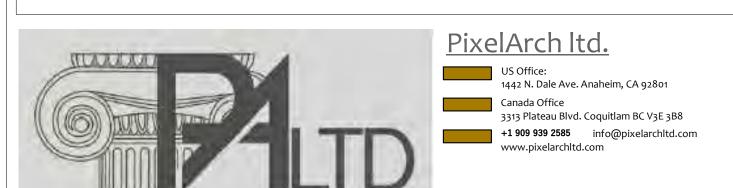


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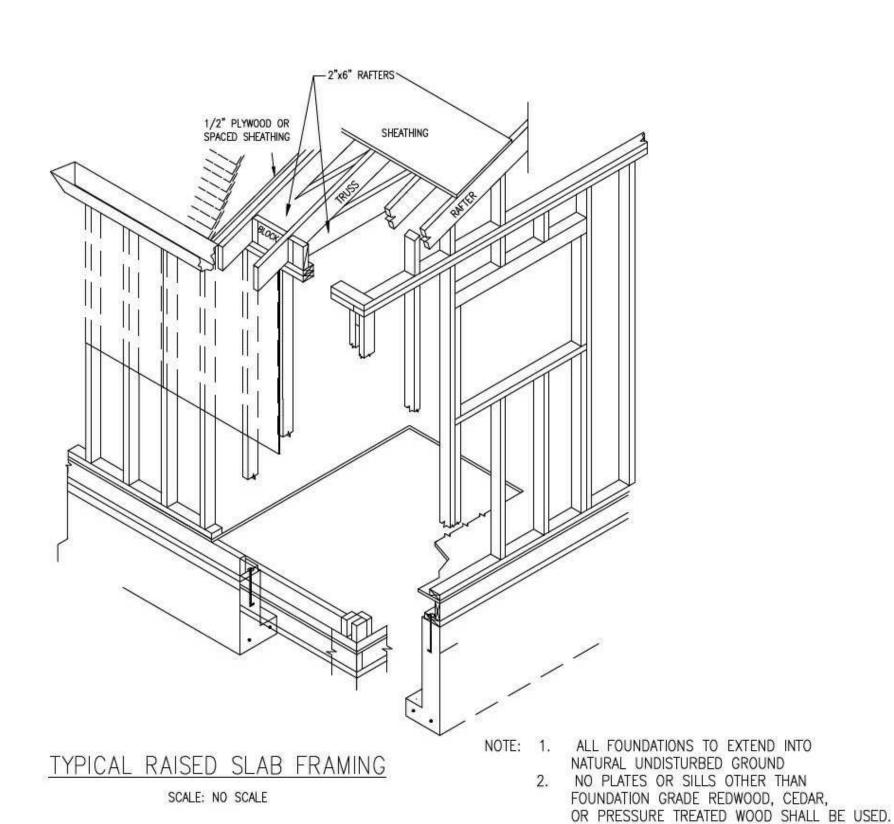
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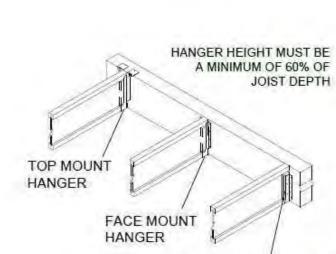




SINGLE FAMILY HOUSE REMODEL 3612 6th AVENUE, LOS ANGELES, CA 90018



WEB STIFFENERS 8" DIAMETER MAXIMUM HOLE FOR 111/8" - 16" DEEP BLOCKING REQUIRED BOTH SIDES AT E1W PANELS; 6" DIAMETER MAXIMUM FOR BLOCKING PANELS 91/2" DEEP OR SHORTER THAN 12" LONG. DO NOT CUT FLANGES. 11/4" TIMBERSTRAND LSL OR ILEVEL 11/8" RIM BOARD CLOSURE, TYPICAL.



WEB STIFFENERS REQUIRED IF SIDES OF HANGER DO NOT LATERALLY SUPPORT AT LEAST 3/8" OF TJI JOIST TOP FLANGE

FLOOR FRAMING & TRUSS NOTES

FLOOR: 40 PSF LL. *10 PSF TOP CHORD DL. FLOORS 3 AND 2 SHALL HAVE 22" FLOOR TRUSSES; BOTTOM FLOOR IS CONCRETE SLAB. SUB-FLOOR SHEATHING SHALL BE H 1.125". SHEATHING IS REQUIRED FOR ANY LONGITUDINAL(DRAG) FORCES. TRUSSES SPACED AT 24.0" O/C. TRUSS DRAWING IS FOR ILLUSTRATION ONLE. ALL TRUSSES SHALL BE INSTALLED & BRACED TO MANUFACTURERS DRAWINGS & SPECIFICATIONS. ALL PLATES ARE 1.5 X 4 UNO DEFLECTION MEETS L/480 LIVE AND L/360 TOTAL LOAD. FASTEN RATED SHEATHING TO ONE FACE OF THIS FRAME. ALL TRUSSES WILL NOT BE FIELD ALTERED WITHOUT PRIOR BUILDING DEPT. APPROVAL OF ENGINEERING CALCULATION.

ALL TRUSSES SHALL HAVE DESIGN DETAILS & DRAWINGS ON SITE FOR FRAMING INSECTION.

NAILING NOTES: (PER IRC TABLE R602.3(1))

TOE NAIL (3)8d JOIST TO SILL OR GIRDER **BRIDGING TO JOIST** TOE NAIL EA, END (2)-8d SOLE PLATE TO JOIST OR BLK'G FACE NAIL 16d @ 16"OC STUD TO SOLE PLATE TOE NAIL (4)-8d, END NAIL (2) 16d TOP PLATE TO STUD END NAIL (2)-16d DOUBLE STUDS FACE NAIL 16d @ 24" OC DOUBLE TOP PLATES FACE NAIL 16d @ 16" OC 16d @ 16" OC ALONG EA. EDGE CONTINUOUS HEADER, TWO PIECES

ALL DLOOR TRUSSES SHALL CARRY MANUFACTURERS STAMP.

BUILT , UP HEADER, TWO PIECES W/ 1/2" SPACER 16d @ 16" OC ALONG EA, EDGE TOP PLATES, LAPS AND INTERSECTIONS FACE NAIL (2)-16d TOE NAIL (3)-8d CEILING JOISTS TO PLATE CONTINUOUS HEADER TO STUD TOE NAIL (3)-8d CEILING JOISTS, LAPS OVER PARTITIONS FACE NAIL (3)-10d CEILING JOISTS TO PARALLEL RAFTERS FACE NAIL (3)-10d RAFTER TO PLATE TOE NAIIL (2)-16d

2" PLANKS (2)-16d @ EA.BRG. 1/2" PLYWOOD SUBFLOOR EDGES 8D @ 6" OC

INTERMEDIATE 8d @ 12" OC

2x MULTIPLE JOISTS , STAGGER @ 15" OC W/(2) @ EA, END OR SPLICE

1" BRACE TO EACH STUD AND PLATE

BUILT , UP CORNERS STUDS

(3) OR FEWER (4) OR MORE 1/2" DIA M.B. W/ STANDARD NUT AND WASHERS

FACE NAIL (2)-8D

10d @ 24" OC

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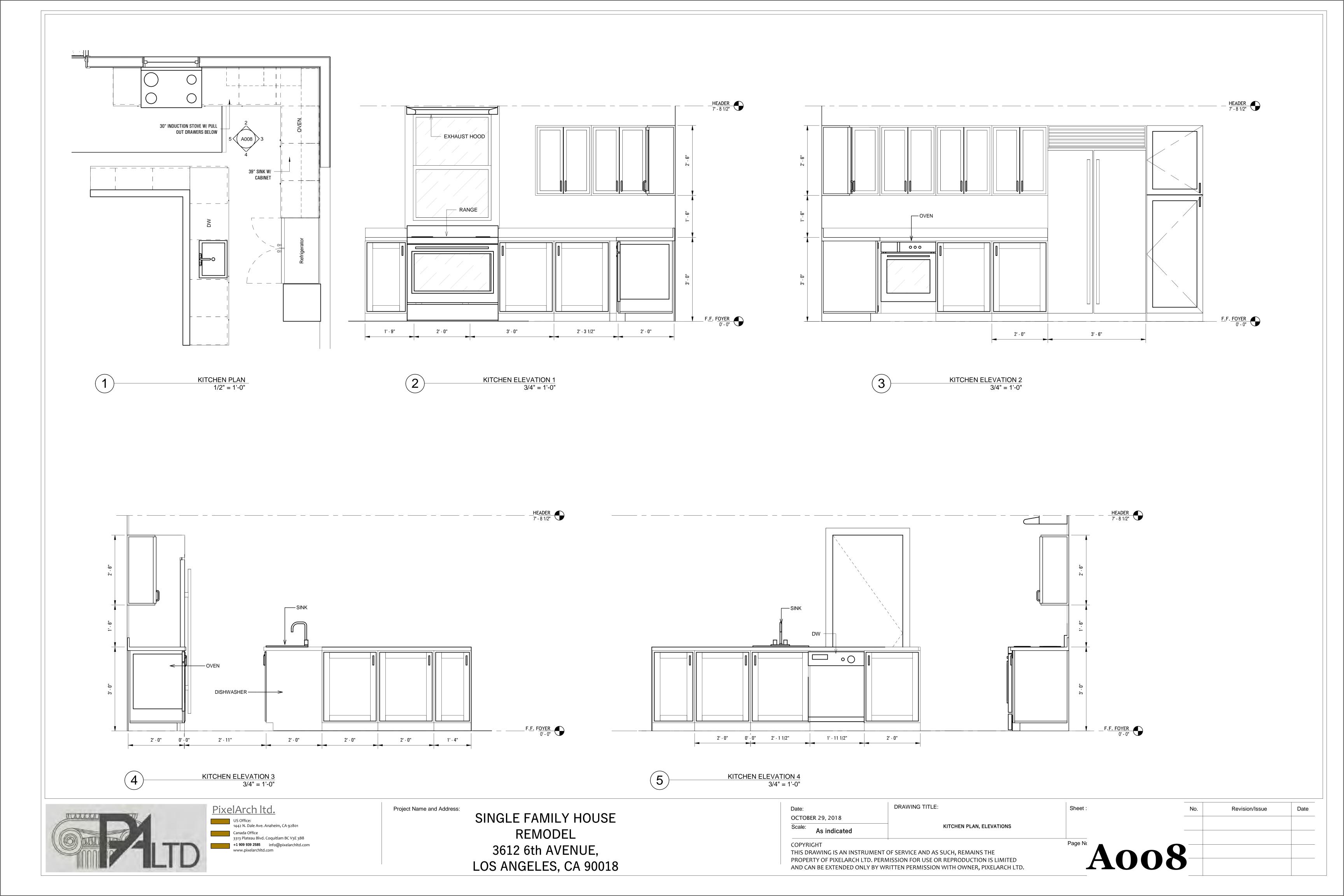
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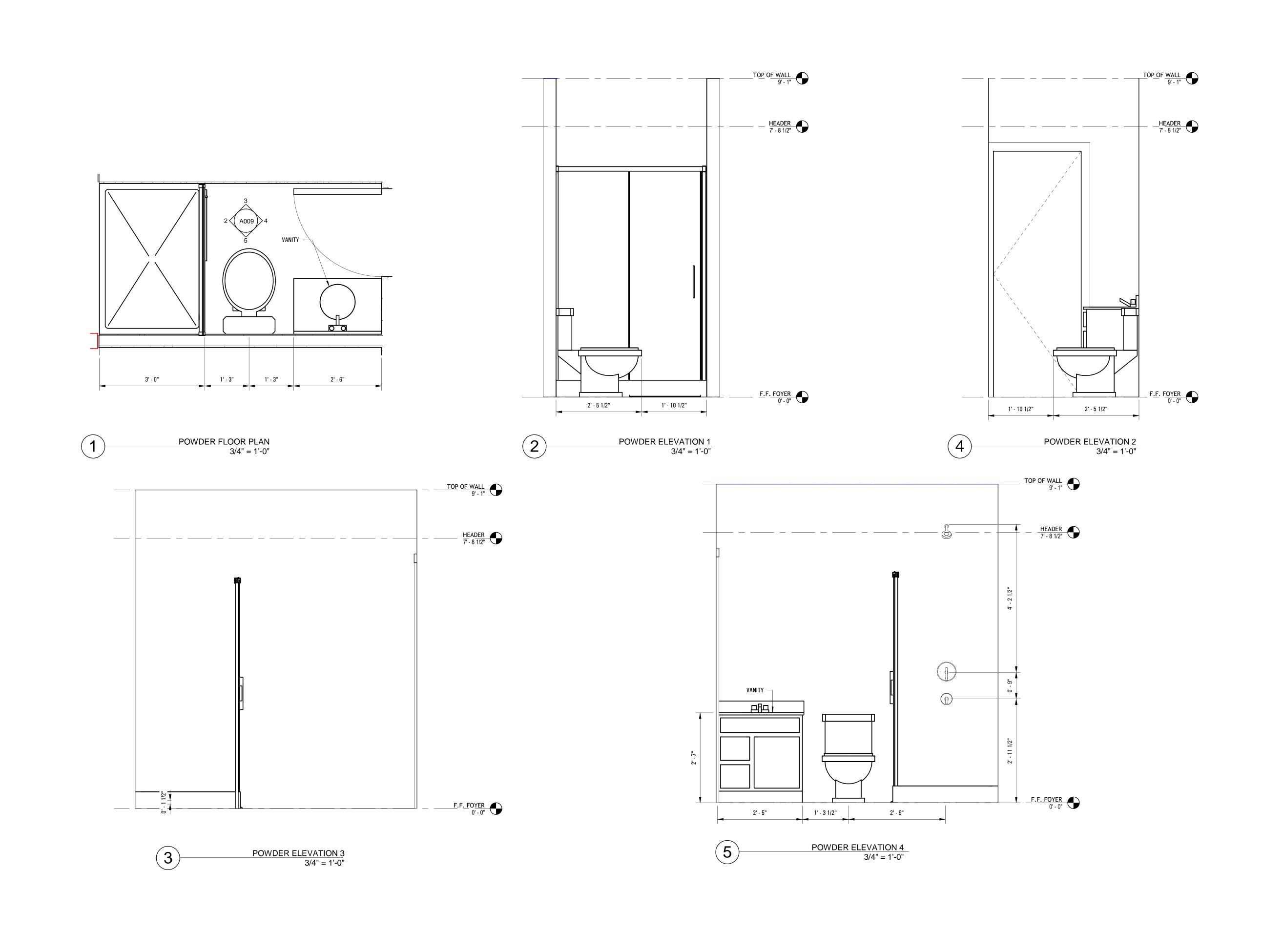
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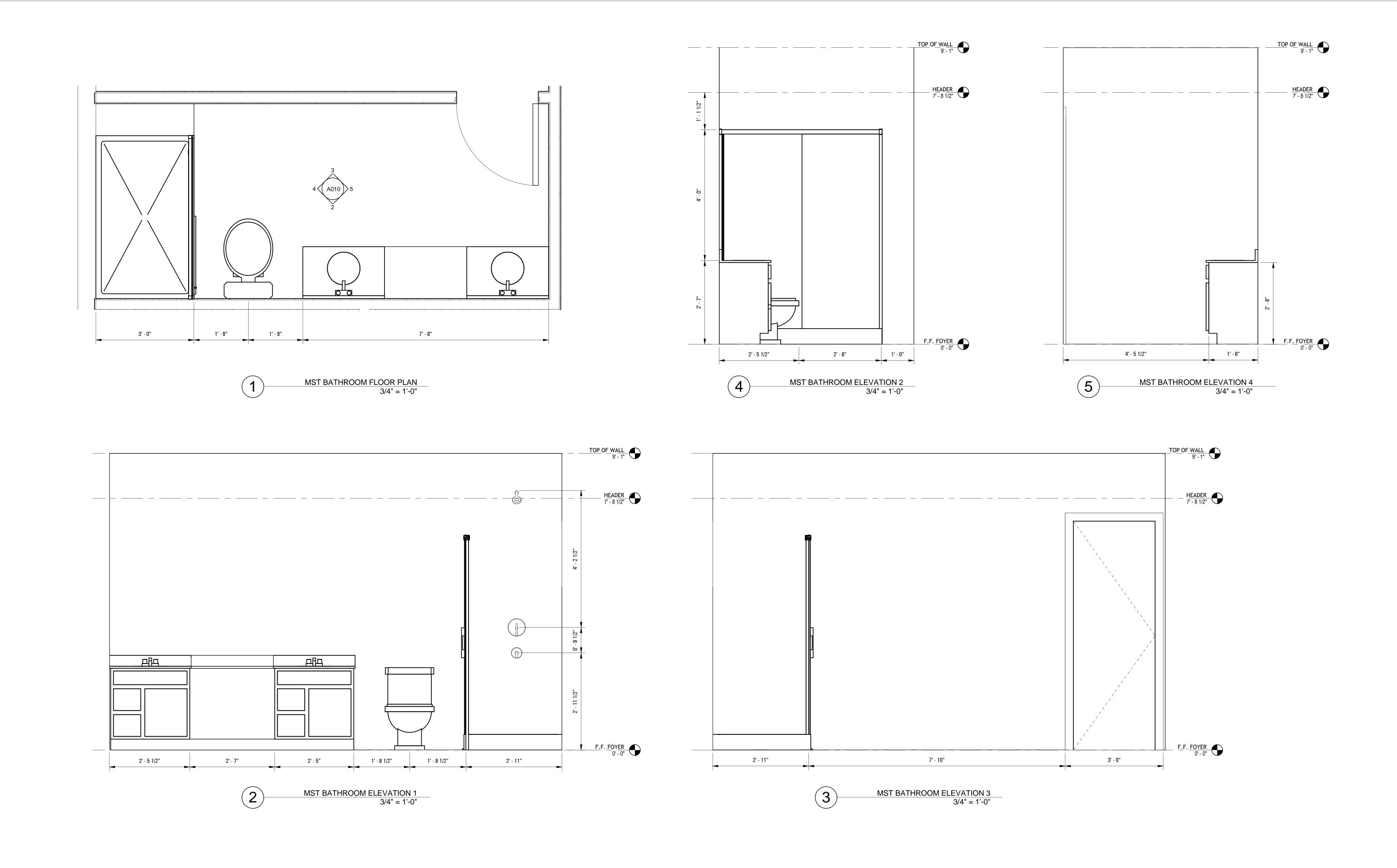
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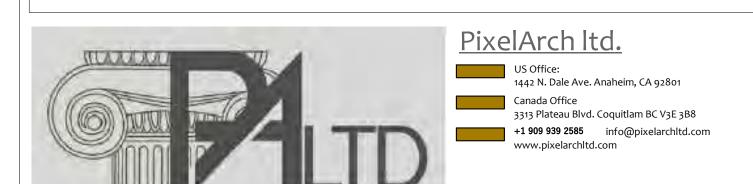




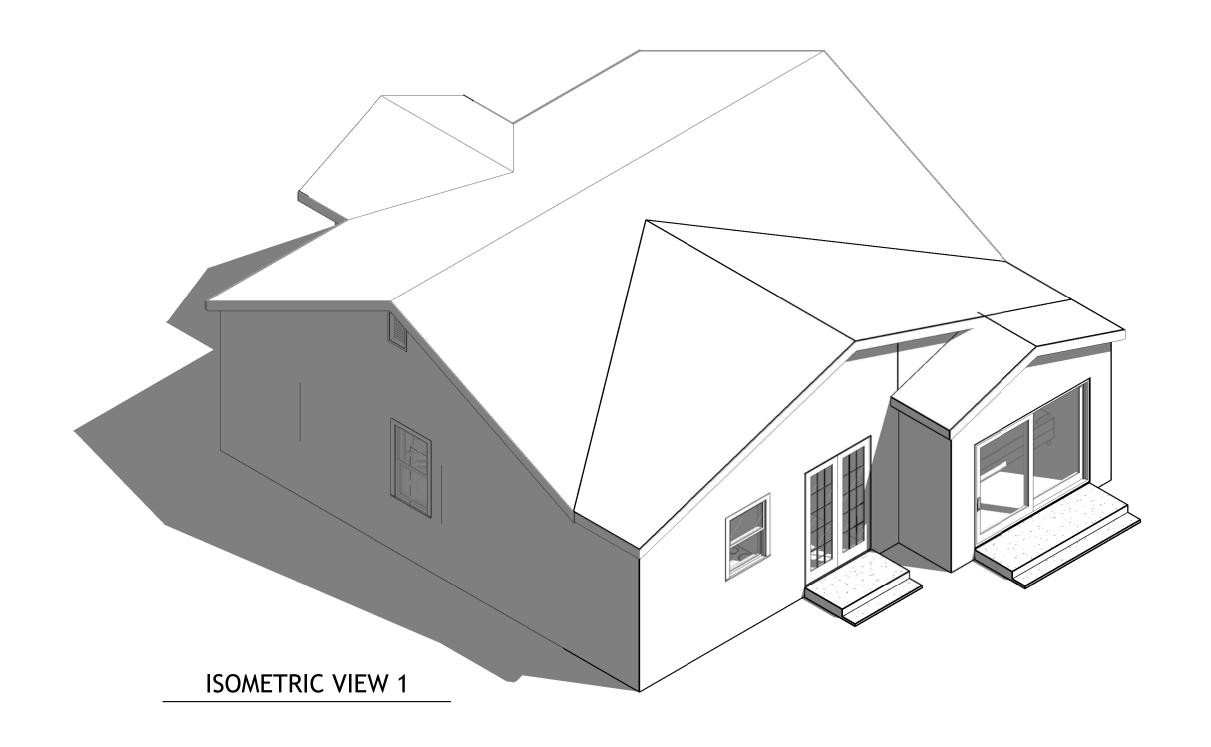


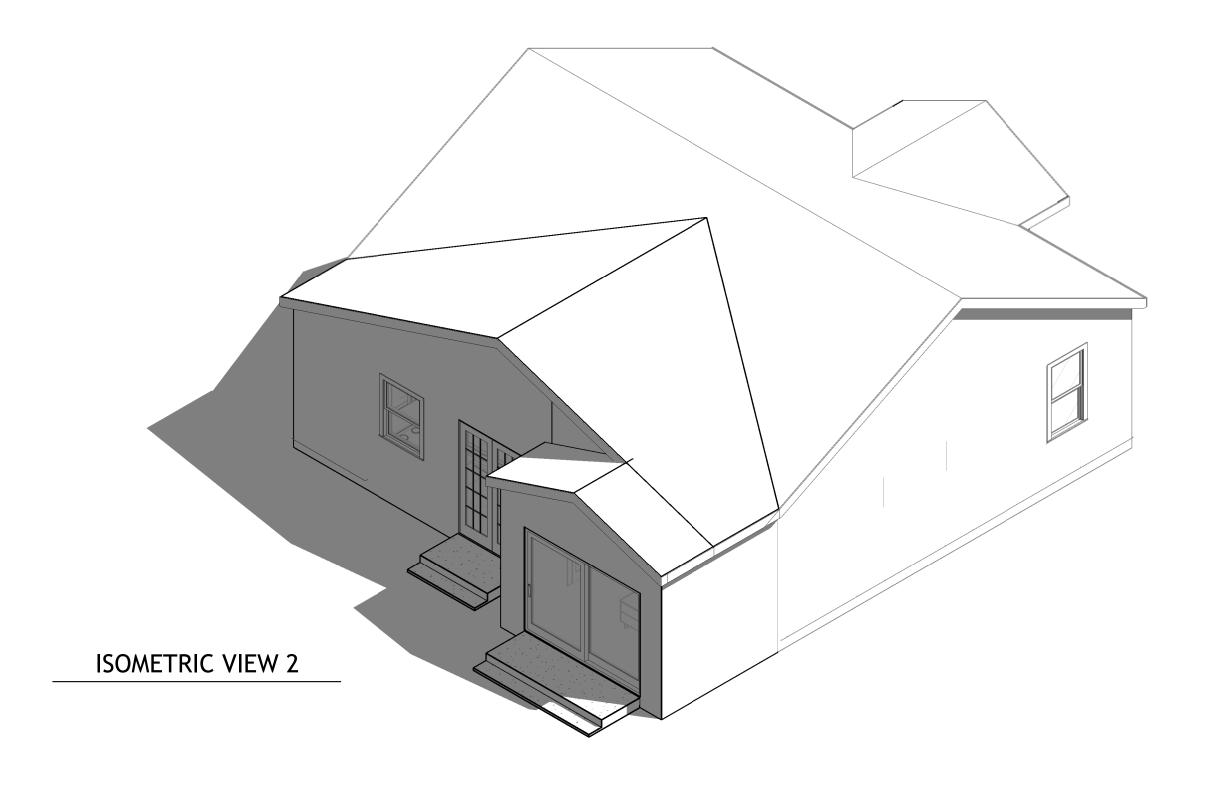
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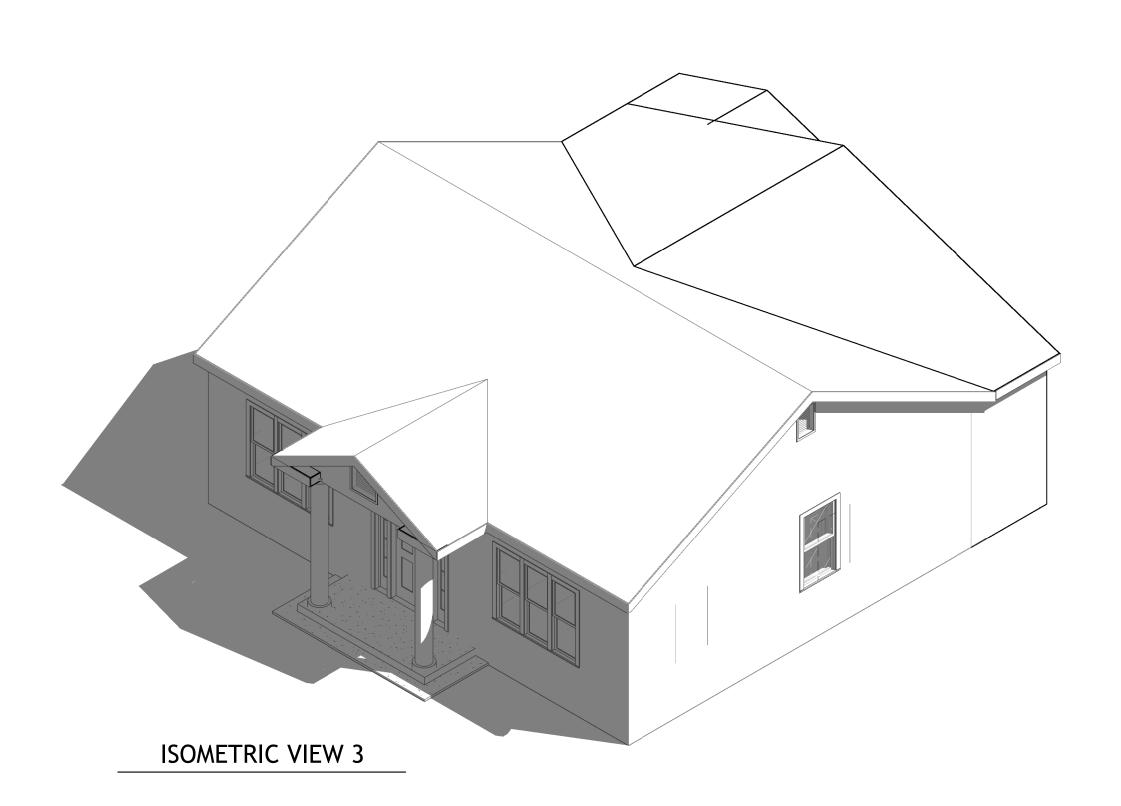


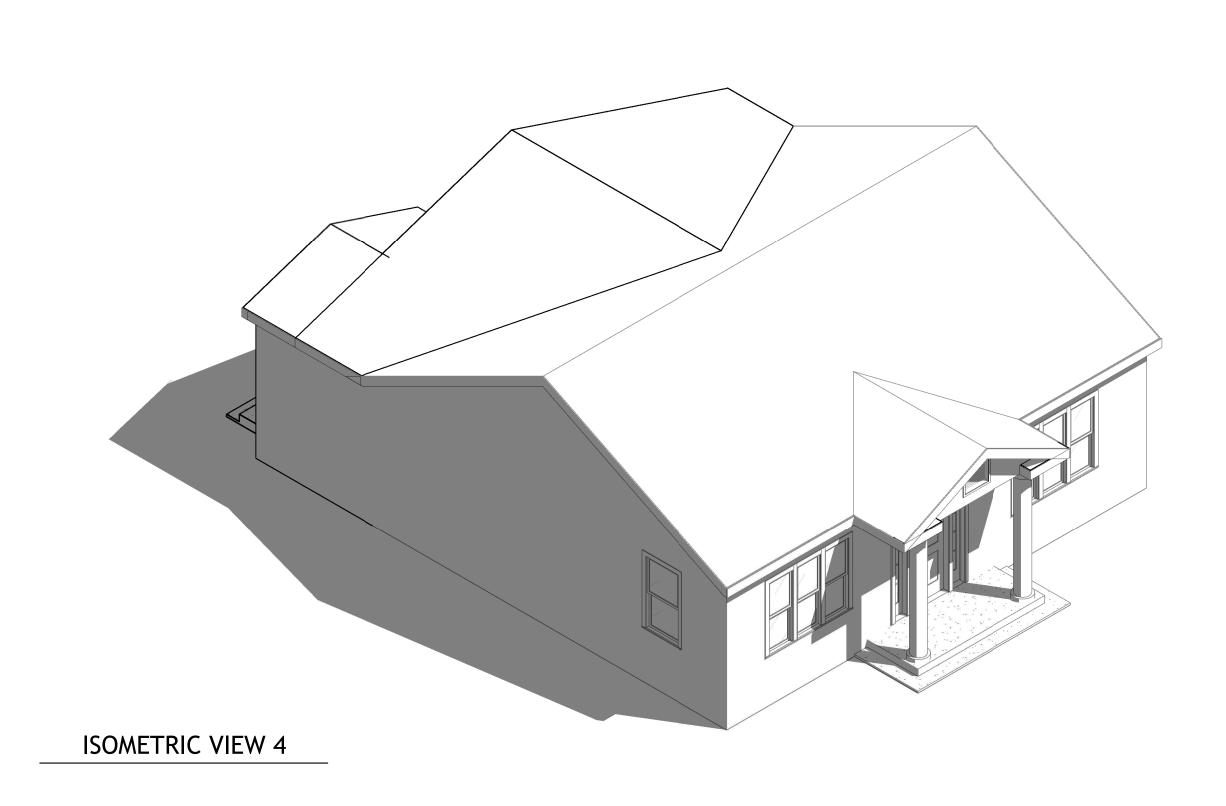


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Revision/Issue

	SHEAR WALL SCHEDULE C.B.C.				
S.W. TYPE	SHEAR PANEL DESCRIPTION	ALLOWABLE SHEAR (PLF)	SILL BOLT'G @ FOUNDATION	TOP PL. TO BLK'G.	SILL NAILING UPPER STORIES
1	15/32" APA RATED PLYWOOD SHT'G. STRUCT I WITH 8d COMMON NAILS @ 6" O.C. AT EDGES & 12" O.C. FIELD	280	5/8" @ 32" O.C.	A35 @ 16"	16d @ 6" O.C.
	(TABLE 2306.4.1 CBC) SEE NOTES 1,2,8,9, AND 10 BELOW.	*560	5/8" @ 16" O.C.	A35 @ 8"	16d @ 3" O.C.
2	15/32" APA RATED PLYWOOD SHT'G. STRUCT I WITH 8d COMMON NAILES @ 4" O.C. AT EDGES & 12" O.C. FIELD	430	5/8" @ 24" O.C.	A35 @ 8"	16d @ 4" O.C.
<u> </u>	(TABLE 2306.4.1. CBC) SEE NOTES 1,2,4,5,8,9, AND 10 BELOW.	*860	5/8" @ 14" O.C.	LTP4 @ 6"	16d @ 2" O.C.
<u></u>	15/32" APA RATED PLYWOOD SHT'G STRUCT I WITH 8d COMMON NAILS @ 3" O.C. AT EDGES & 12" O.C. FIELD	550	5/8" @ 20" O.C.	A35 @ 8"	16d @ 3" O.C.
<u>/</u>	(TABLE 2306.4.1. CBC) SEE NOTES 1,2,4,5,8,9, AND 10 BELOW.	*1100	3/4" @ 16" O.C.	LTP4 @ 6"	1/4"□ X 3-1/2" LAG SC. @ 2" O.C.
4	15/32" APA RATED PLYWOOD SHT'G. STRUCT I WITH 8d COMMON NAILS @ 2" O.C. AT EDGES & 12" O.C. FIELD	730	5/8" @ 16" O.C.	A35 @ 8"	16d @ 2-1/2" O.C.
/ * \	(TABLE 2306.4.1. CBC) SEE NOTES 1,2,4,5,8,9, AND 10 BELOW.	*1460	3/4" @ 16" O.C.	LTP4 @ 6"	1/4" □ X 3-1/2" LAG SC. @ 2" O.C.
5	15/32" APA RATED STRUCT. I SHT'G. WITH 10d COMMON NAILS @ 2" O.C. AT EDGES & 12" O.C. FIELD	870	3/4" @ 16" O.C.	A35 @ 6"	#12 X 3-1/2" WD. SC. @ 2" O.C.
<u> </u>	OVER 3 X STUDS (TABLE 2306.4.1 CBC) SEE NOTES 1,4,5,8,9, AND 10 BELOW.	*1740	3/4" @ 8" O.C.	LTP4 @ 4-1/2"	1/4" □ X 3-1/2" LAG SC. @ 1-1/2" O.C.

NOTES:

- . ALL EDGES OF PLYWOOD SHEAR WALLS MUST BE BLOCKED WITH 2X SOLID BLOCKING.
- 2. DESIGNATES SILL BOLTING OR NAILING WHERE SHEAR WALL PANELS ARE TO BE APPLIED TO BOTH SIDES OF
- B. PAPER BACKED SELF-FURRING EXPANDED METAL OR WOVEN WIRE LATH AND PORTLAND CEMENT PLASTER.
- 4. FRAMING AT ADJOINING PANEL EDGES SHALL BE 3-INCH NOMINAL OR WIDER AND NAILS SHALL BE STAGGERED. (USE 3X SILL PLATE @ FOUND., FOR SHEAR LOADS LESS THAN 350 PLF 2X SILL PLATE MAY BE USED.)
- 5. WHERE PANELS ARE APPLIED ON BOTH FACES OF A WALL AND NAILS SPACING IS LESS THAN 6" O.C. ON EITHER SIDE, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS OR FRAMING SHALL BE 3" NOMINAL OR THICKER & NAILS ON EACH SIDE SHALL BE STAGGERED. (USE 3 X SILL PLATE @ FOUND.)
- 6. ALL CONTINUOUS EXTERIOR AND INTERIOR SHEAR/BEARING WALL FOOTINGS TO HAVE 5/8" A.B.'S @ 48" O.C. WITH 3" X 3" X 1/4" PLATE WASHERS U.N.O.
- MINIMUM OF TWO BOLTS PER EACH PIECE OF SILL PLATE AT 4" TO 12" CLEARANCE TO THE END AND 7" MINIMUM EMBEDMENT.(FOR TWO POUR SYSTEM,BOLTS SHALL BE EMBEDDED 4 INCH MIN. INTO FIRST POUR.) (SEE NOTE 12 FOR A.B. LENGTH.)
- 7. ALL INTERIOR NON-BEARING FTGS TO HAVE 3/16"

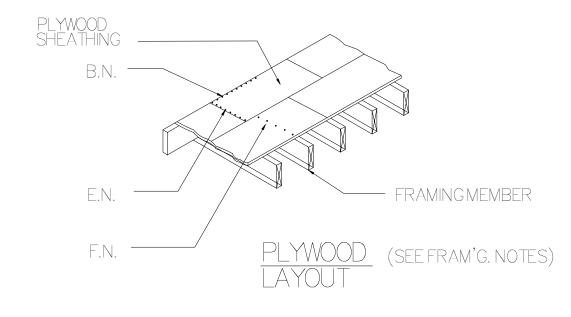
 SHOT PINS AT 32" O.C., I.E., HILTI SHOT PINS (ICC ESR-1663).
- 8. USE APA RATED PLYWOOD SHEATHING. OR O.S.B. PANEL. ALL PLYWOOD SHALL BE DOUGLAS FIR. 4-PLY MIN. OTHER SPECIES MAY REQUIRE
- 3 X 3 X 5/16 PLATE WASHERS WITH 3/4" ☐ A.B. AT ALL SHEAR WALLS. 10. AT EXISTING FOOTINGS, USE THREADED RODS W/ SIMPSON "SET-XP"

EPOXY 7" MIN. EMB. W/MIN. EDGE DIST. OF 1-7/8" (ICC ESR-2508) (SPECIAL

9. USE 3 X 3 X 1/4 PLATE WASHERS WITH 5/8" A.B. AT ALL SHEAR WALLS. USE

11. ALL ANCHOR BOLTS SHALL CONFORM TO ASTM A-307 U.N.O.

INSPECTION REQ'D.)



NAILING: (EXCEPT WHERE NOTED OTHERWISE)

	ROOF NAIL'G	FLOOR NAIL'G
B.N. = BOUNDARYNAILING	8d @ 6" O.C.	10d @ 6" O.C.
E.N. = EDGE NAILING	8d @ 6" O.C.	10d @ 6" O.C.
F.N. = FIELD NAILING	8d@12"O.C.	10d @ 10" O.C.

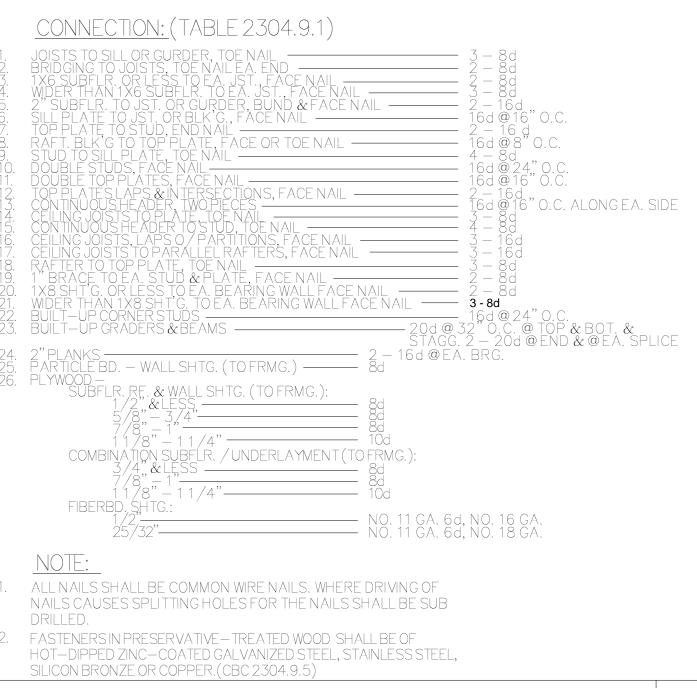
1. NAILS SHALL BE GALV. COMMON(HOT-DIPPED OR TUMBLED), PLACED NOT LESS THAN 3/8" FROM PANEL EDGES AND SHALL BE FIRMLY DRIVEN.

2. NO UNBLOCKED PIECE LESS THAN 12" SHALL BE USED.

3. WOOD STRUCTURAL PANELS SHALL COMPLY WITH 2010 CBC STANDARD AND SHALL BE APA

4. WOOD STRUCTURAL PANELS, WHEN USED, SHALL COMPLY WITH THE REQUIREMENTS FOR THEIR TYPE IN DOC PSI-95 OR PS2-92.

5. ALL PANELS SHALL BE IDENTIFIED BY TRADE MARK OF AN APPROVED TESTING & GRADING AGENCIES, APA, TECO OR PITTSBURG.



SHEAR WALL SCHEDULE

DESIGN DATA:
APPLICABLE DESIGN LOADS: PER ASCI/SEI 7-10
FLOOR DEAD LOAD: 15PSF FLOOR LIVE LOAD: 40 PSF ROOF LIVE LOAD: 20 PSF BASIC WIND SPEED: 85 MPH EXPOSURE: D STRUCTURAL CATEGORY: II SEISMIC DESIGN CATEGORY = D L PRESSURES SHOWN ARE BASED ON ASD DESIGN ,

GENERAL NOTES:

- . CONTRACTOR TO ASSUME FULL RESPONSIBILITY FOR ABIDING TO ALL APPLICABLE CALIFIORNIA BUILDING CODES LOCAL CITY ORDINANCES, ZONING REQUIREMENTS, AND LICENSING/PERMIT REQUIREMENTS. CONTRACTOR IS FULLY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES INCLUDING WITHOUT LIMITATION TO DEMOLITION, EXCAVATION AND ERECTION PROCEDURES.
- THE CONTRACTOR SHALL EXAMINE THE CONSTRICTION DOCUMENTS AND NOTIFY THE PROJECT ENGINEER & ARCHITECT OF ANY DISCREPANCIES, ERRORS, OR OMISSIONS SHE/HE MAY FIND BEFORE PROCEEDING WITH THE
- . NOTIFY THE PROJECT ENGINEER OF ANY DESIGN CHANGES PROPOSED BY OWNER OR THE CONTRACTOR. DURING THE COURSE OF CONSTRUCTION. SUCH CHANGES AFFECTING ROOM ADDITION DESIGN MAY ALSO AFFECT
- ANY SUBCONTRACTOR WHICH AGREES TO CONSTRUCT THE PROJECT PURSUANT TO THESE PLANS FULLY ASSUMES THE RISK OF ALL ERRORS AND OMISSIONS WHICH SHOULD HAVE BEEN DETECTED BY A CAREFUL REVIEW BY A KNOWLEDGEABLE LICENSED CONTRACTOR, THAT WHICH FOR ANY REASON WERE NOT RESOLVED DURING THE THE WORK PROGRESSES IN ORDER TO IDENTIFY ANY SIGNIFICANT ERRORS AND OMISSIONS AND TO ASCERTAIN ALL NECESSARY INFORMATION BEFORE PROCEEDING WITH THE AFFECTED WORK, AND ASSUMES THE RISK OF
- NY AND ALL LOSS, INCLUDING DELAY, WHICH MAY BE CAUSED OR CONTRIBUTED TO BY THE FAILURE TO ASCERTAIN CORRECT OR NECESSARY INFORMATION IN A TIMELY MANNER. ALL TRADES SHALL AT ALL TIMES. KEEP THE PREMISES FREE FROM ACCUMULATION OF WASTE MATERIALS OR RUBBISH CAUSED BY THEIR WORK, AND AT THE COMPLETION OF THE WORK SHALL REMOVE ALL RUBBISH FROM AND ABOUT THE JOBSITE AND ALL THEIR TOOLS, SCAFFOLDING AND SURPLUS MATERIALS, AND SHALL LEAVE THE

JOB BROOM CLEAN, INCLUDING REMOVING ALL LABELS, STICKERS, PAINT SMEARS, ETC..., FROM LIGHTING

6. EXCEPT WHERE MORE STRINGENT REQUIREMENTS ARE NOTED OR SHOWN ON THE PLANS, WORKMANSHIP &

MATERIALS SHALL CONFORM. TO THE LATEST EDITION OF THE C.B.C. OR LOCAL CODE.

- 7. THE PLANS SHALL BE REVIEWED FOR DIMENSIONAL & EXISTING SITE CONFORMANCE WITH THE PLANS BY THE CONTRACTOR PRIOR TO THE START OF CONSTRUCTION. THE ARCHITECT & ENGINEER SHALL BE NOTIFIED OF ANY
- 3. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS IN THE FIELD: AND ALL QUESTIONS AS TO DIMENSIONS AND FIELD CONDITIONS SHALL BE RESOLVED BEFORE THE AFFECTED WORK PROCEEDS. NO
- DIMENSIONS SHALL BE OBTAINED BY SCALING THESE PLANS. 9. CONTRACTORS SHALL VERIFY AND BE RESPONSIBLE FOR DIMENSIONS AND CONDITIONS OF THE JOB.
- 10. THE PRECISE DIMENSIONS AND LOCATIONS OF ALL DOOR, WINDOW AND ROOF OPENINGS SHALL BE DETERMINED FROM DRAWINGS AND OTHER FLOOR, WALL OPENING REQUIRED BY MECHANICAL OR ELECTRICAL SHALL BE VERIFIED FROM SHOP DRAWINGS, EQUIPMENT DATA SHEETS, ETC. AS REQUIRED.
- 11. ITEMS IDENTIFIED BY TRADE NAMES MAY BE SUBSTITUTED BY APPROVED EQUALS. 12. NOTES & DETAILS ON DRAWINGS SHALL PRECEDE THESE GENERAL NOTES.
- 13. PROVIDE ANY SHORING & OR BRACING PRIOR TO REMOVING EXISTING WALLS. BEAMS, OR SUPPORTS FOR CONSTRUCTION. REMOVE SHORING ONLY WHEN NEW SUPPORTS ARE IN PLACE AND SECURED.
- 14. PROVIDE RED HEADS INTO EXISTING CONCRETE AT ALL SHEAR WALLS PER MFG. SPECIFICATIONS. SEE SHEAR WALL SCHEDULE FOR SIZE AND SPACING.
- 15. PROVIDE SIMPSON ST-6224 BETWEEN NEW WALLS AND EXISTING WALLS AT THE DOUBLE TOP PLATE.
- 16. THE CONTRACTOR SHALL LOCATE ALL EXISTING UTILITIES WHETHER OR NOT SHOWN ON DRAWINGS AND PROTECT THEM FROM DAMAGE
- 17. DO NOT CUT POST TENSION SLABS. CONTRACTOR TO DETERMINE EXISTING CONDITIONS PRIOR TO START OF CONSTRUCTION.
- 18. THE CONTRACTOR SHALL VERIFY ALL EXISTING CONDITIONS FOR FOOTING, BEAMS AND JOISTS, SIZES, LOCATIONS, ETC., AND SHALL NOTIFY THE ARCHITECT & ENGINEER OF ANY DISCREPANCIES.
- 19. DOWEL NEW INTO EXISTING SLABS W/#4 REBAR @ 24" O.C. AND FOOTINGS W/ DOWELS TO MATCH NEW REINF. SIZE/

ENGINEERING NOTES

- CONCRETE SLABS ON GRADE HAVE NOT BEEN DESIGNED BY THE STRUCTURAL ENGINEER.
- . THE VIBRATIONAL EFFECTS OF MECHANICAL EQUIPMENT HAVE NOT BEEN CONSIDERED BY THE STRUCTURAL THE DESIGN ADEQUACY AND SAFETY OF ERECTION, BRACING SHORING, TEMPORARY, SUPPORTS FTC. IS THE
- ALLOWABLE SOILS PRESSURE TO BE A MINIMUM OF 1500 PSF UNLESS A SOILS REPORT IS PROVIDED. SOILS IN THE BUILDING AREA & 5 FEET BEYOND SHALL BE COMPACTED TO A MINIMUM OF 90% RELATIVE COMPACTION PER

SOLE RESPONSIBILITY OF THE CONTRACTOR, AND HAS NOT BEEN CONSIDERED BY THE STRUCTURAL ENGINEER.

HE CONTRACTOR IS RESPONSIBLE FOR THE STABILITY OF THE STRUCTURE DURING THE ENTIRE COURSE OF

CONSTRUCTION. THE ENGINEER SHALL NOT BE HELD RESPONSIBLE FOR FIELD INSPECTION/OBSERVATION OF

REFER TO SOIL REPORT BY

JOB NUMBER STRUCTURAL SYMBOLS

INDICATES SHEAR WALL. SEE FOUNDATION, FRAMING PLAN AND SHEAR WALL SCHEDULE FOR TYPE, SILL BOLTING, SHTG.. ETC. NOTE: FOR SILL BOLTING AT EXISTING FOOTINGS, USE 5/8"

THREADED RODS W/ SIMPSON "SET-XP" EPOXY 7" MIN. EMB. PER ICC ESR-2508 THE SAME SIZE & SPACING AS CALLED FOR ON PLANS

INDICATES POST (BELOW BEAM)

MIN. POST SIZE/TYPE AS FOLLOWS U.N.O.

4 X 12 & SMALLER 2-2X4 W/16d NAILS @ 12" O.C. 4 X 14 & LARGER

6 X 12 & LARGER 6X6 SEE HOLDDOWN DETAILS AND TYPICAL WALL FRAMING FOR FURTHER POST SIZE REQUIREMENTS. POSTS ARE TO CONTINUE DOWN TO FOUNDATION

FOUNDATION NOTES <u>GENERAL</u>

. SOIL BENEATH FOOTINGS AND SLABS SHALL BE COMPACTED PER 2010 C.B.C. (90%) RELATIVE COMPACTION

- 2. CONTINUOUS FOOTINGS AND GRADE BEAMS SHALL BE EXCAVATED TO THE DEPTH SHOWN ON THE DRAWINGS BELOW UNDISTURBED SOIL OR COMPACTED EARTH. PROVIDE 1-#4 HORIZONTAL BARS ON TOP AND BOTTOM U.N.O. ON FOUND. PLAN.
- 3. ALLOWABLE SOIL BEARING PRESSURE IS ASSUMED TO BE 1500. PSF IF NO SOILS REPORT IS PROVIDED. I. SLAB ON GRADE: 4 INCH. NET CONCRETE SLAB WITH #3 BARS @ 18" O.C. EACH @ CENTER OF SLAB OVER 2 INCH. OF SAND OVER 6 MIL. VISQUEEN OVER 2" SAND BED OVER COMPACTED SOIL. U.N.O.
- 5. NO TRENCHES OR EXCAVATIONS FIVE FEET IN DEPTH OR GREATER INTO WHICH A PERSON SHALL BE REQUIRED TO DESCEND SHALL BE MADE WITHOUT PROPER PERMIT. . THE MINIMUM BOLTING FOR SILL PLATES TO FOUNDATION SHALL BE AS FOLLOWS: 5/8" DIAMETER ANCHOR BOLTS WITH 7" MIN EMBEDMENT IN CONCRETE WITH SPACING NO GREATER THAN 4 FEET O.C. NOR FURTHER THAN 12" FROM CORNERS (MIN 2 BOLTS PER PIECE). SEE THE FOUNDATION PLAN &
- 7. PIPES OR DUCTS THAT EXCEED ONE THIRD THE SLAB OR CONC. WALL THICKNESS SHALL NOT BE PLACED IN STRUCTURAL CONC. UNLESS SPECIFICALLY DETAILED. SEE MECHANICAL AND/OR ELECTRICAL DRAWINGS FOR LOCATION OF SLEEVES, ACCESSORIES, ETC.

SHEAR WALL SCHEDULE FOR FURTHER BOLTING REQUIREMENTS. (FOR TWO POUR SYSTEMS, BOLTS

3. PIPES MAY PASS THRU STRUCTURAL CONC. IN SLEEVES, BUT SHALL NOT BE EMBEDDED THEREIN. 9. PROVIDE 3/4" CAMBERS AT ALL EXPOSED CORNERS.

SHALL BE EMBEDDED 4 INCH MIN. INTO FIRST POUR.)

0. SEE ARCHITECTURAL PLANS FOR MOLDS, GROOVES, ORNAMENTS, CLIPS OR GROUNDS REQUIRED TO BE CAST IN CONCRETE, AND FOR LOCATION OF FLOOR FINISHES AND SLAB DEPRESSIONS 1. LOCATION OF POUR JOINTS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER.

- 1. UNLESS OTHERWISE NOTED ON PLANS, CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 2500 PSI IN 28 DAYS
- 2. FINE & COURSE AGGREGATE SHALL CONFORM TO A.S.T.M. C-33. USE 3000 P.S.I. CONC. @ GRADE BEAMS, CEMENT SHALL CONFORM TO A.S.T.M. C-150 (STANDARD BRAND PORTLAND CEMENT) TYPE II (USE TYPE V CEMENT IF NOTED IN SOILS REPORT)
- 3. CONCRETE SHALL BE MACHINE-MIXED USING A MAXIMUM OF '7' GALLONS OF WATER PER SACK OF CEMENT. READY MIX CONCRETE SHALL BE MIXED AND DELIVERED IN ACCORDANCE WITH ASTM C-94 MIXED AT A RATE OF 5 SACKS OF CEMENT PER CUBIC. YARD, MAXIMUM SI UMP SHALL BE 4 INCH AS MEASURED BY THE ASTM "STANDARD METHOD OF TESTING FOR SLUMP OF PORTLAND CEMENT
- 4. DRY PACK SHALL CONSIST OF 1 PART CEMENT, 4 PARTS SAND. BASED ON DRY LOOSE VOLUMES AND NOT LESS THAN 1/4 PART, NOR MORE THAN 1/2 PART, LINE PUTTY OR DRY HYDRATED LIME. DRY PACK SHALL OBTAIN A MINIMUM ULTIMATE COMPRESSIVE STRENGTH OF 2000 PSI AT 28 DAYS. 5. ADDING CALCIUM CHLORIDE TO CONCRETE OR GROUT IS NOT PERMITTED.
- 6. CONC. SHALL BE KEPT MOIST FOR 10 DAYS FOR PROPER CURING.

EQUIREMENTS FOR CONCRETE EXPOSED TO SULFATE-CONTAINING SOLUTIONS (ACI 4.3 WATER-CEMENT RATIO STRENGTH NEGLIGIBLE NOT REGULATED 2500 psi MODERATE 4000 psi

CONCRETE BLOCK MASONRY

- I. CONCRETE BLOCK SHALL CONFORM TO A.S.T.M. C-90 MED. WT. GRADE N UNITS. WITH MIN. COMP. STRENGTH OF 1500 PSI. ALL CMU BLOCKS SHALL BE LAID UP IN RUNNING OR COMMON BOND
- 2. MORTAR SHALL CONFORM TO ASTM C-270, TYPE S, WITH MINIMUM COMPRESSIVE STRENGTH OF

MIX: 1 PART PORTLAND CEMENT. 1/2 PART LIME PUTTY. BY VOLUME 4 PARTS SAND.

3. GROUT SHALL CONFORM TO ASTM C-476, WITH MINIMUM COMPRESIVE STRENGTH OF 2000. PSI AT

MIX: 1 PART PORTLAND CEMENT. BY VOLUME 3 PARTS SAND 2 PARTS PEA GRAVEL

WATER SUFFICIENT TO ALLOW GROUT TO FLOW INTO ALL JOINTS.

STOPPING GROUT 1-1/2 INCH BELOW THE TOP OF THE BLOCK.

- 4. CELLS SHALL BE IN VERTICAL ALIGNMENT TO PROVIDE A MIN. UNOBSTRUCTED CORE OF 3" X 3" DOWELS FROM FOOTINGS SHALL BE SET TO ALIGN WITH CORE REINFORCING. 5. ALL CELLS BELOW FINISHED GRADE AND ALL CELLS WITH REINFORCING, ANCHORS OR INSERTS
- SHALL BE FILLED SOLID WITH GROUT. 6. CONCRETE SURFACES SHALL BE CLEANED OF ALL LAITANCE PRIOR TO SETTING OF BLOCKS.
- 7. PROVIDE VERTICAL CONSTRUCTION JOINTS AT 40 FT. O.C. 3. MINIMUM LAP FOR ALL STEEL IS 40 BAR DIAMETER, OR 24 INCHES, WHICHEVER IS GREATER.

eta. IF WORK IS STOPPED FOR ONE HOUR OR LONGER, PROVIDE HORIZONTAL CONSTRUCTION $\;\;$ JOINTS BY

IO. CLEANOUT OPENINGS MUST BE PROVIDED IN CMU WALLS WHICH ARE TO BE HIGH LIFT GROUTED. (OVER 4 FEET).AT BOTTOM COURSE. IN SOLIÓ GROUTED WALLS CLEANOUTS MUST BE PROVIDED AT NO MORE THAN 32 INCHES ON CENTER FOR PARTIALLY GROUTED WALLS CLEANOUTS SHALL BE PROVIDED NO MORE THAN 48 INCHES O.C. WHEN

CLEANOUTS ARE REQUIRED. THEY SHALL BE SEALED AFTER INSPECTION AND BEFORE GROUTING

REINFORCING STEEL

1. REINFORCING STEEL, #3 AND #4 GRADE 40, #5 AND LARGER GRADE 60 PER A.S.T.M. A615.

2. LOW HYDROGEN WELDING RODS SHALL BE USED FOR ALL WELDING OF REINFORCING BARS. 3. BARS NOTED AS "CONT" TYPICAL WALL REINFORCING AND VERTICAL COLUMN REINFORCING SHALL HAVE A

12. ANCHOR BOLT SPEC.

3X SILL

BOLT LENGTH

12" 16"

14"

SINGLE POUR DOUBLE POUR

- MINIMUM SPLICE OF 50 BAR DIAMETERS LAP IN MASONRY OR 40 BAR DIAMETERS MINIMUM IN CONCRETE. 4. REINFORCING SHALL BE SPLICED ONLY AS SHOWN OR NOTED. OTHER SPLICES SHALL BE APPROVED BY THE STRUCTURAL ENGINEER.
- 5. SPLICES IN ADJACENT HORIZONTAL WALL REINFORCING BARS SHALL BE STAGGERED 4 FEET UNLESS OTHERWISE NOTED.
- 6. PROVIDE DOWELS IN FOOTINGS AND/OR GRADE BEAMS THE SAME SIZE AND NUMBER AS VERTICAL WALL OR COLUMN REINFORCING. DOWELS SHALL HAVE A MINIMUM PROJECTION EQUAL TO STANDARD LAP SPLICE
- 7. ALL REINFORCING, ANCHOR BOLTS, AND OTHER INSERTS SHALL BE SECURED IN PLACE PRIOR TO PLACEMENT OF CONCRETE OR GROUTING OF MASONR 8. PROVIDE THE FOLLOWING MINIMUM PROTECTIVE COVERING OF CONCRETE:
- BELOW GRADE (UNFORMED) 3" CLEAR BELOW GRADE (FORMED) 2" CLEAR WALLS COLUMNS 1.5" CLEAR BEAMS AND GIRDERS 1.5" CLEAR STRUCTURAL SLAB

(ABOVE GRADE) 3/4" CLEAR

9. #5 OR LARGER REINFORCING BARS SHALL NOT BE RE-BENT WITHOUT APPROVAL OF THE STRUCTURAL

GRADING NOTES

1. A GRADING PERMIT SHALL BE OBTAINED PRIOR TO ANY GRADING.

2. ALL FILL ONE FOOT & GREATER SHALL BE CERTIFIED AND TESTED AS TO RELATIVE COMPACTION PER U.B.C. 3. ALL FILL SHALL BE COMPACTED IN ACCORDANCE WITH ASTM D-1557, TO MAXIMUM OF 90% DENSITY. 4. ALL UTILITY TRENCH BACKFILLS SHALL BE IN ACCORDANCE WITH THE SOILS ENGINEER'S

STRUCTURAL STEEL

- . STRUCTURAL STEEL SHALL CONFORM TO ASTM A36,(Fy=36.KSI) FOR PLATES AND TO ASTM
- A992,(Fy=50.KSI) FOR W-SHAPE STEEL SECTIONS. 2. CORTEN STEEL SHALL CONFORM TO ASTM A588, Fy=50. KSI.

TENSILE BOLTS SHALL CONFORM TO ASTM A325 OR A490.

- 3. STAINLESS STEEL SHALL CONFORM TO ASTM A276 TYPE 304-HOT ROLLED, Fy=18. KSI. 4. FABRICATION, ERECTION & PAINTING SHALL COMPLY WITH THE AISC SPECS. CHAPTER M.
- 5. ALL BOLTS FOR STEEL MEMBERS SHALL CONFORM TO ASTM A325 OR A490, UNLESS OTHERWISE NOTED. 6. HIGH TENSILE BOLTS WHERE INDICATED ON THE PLANS OR DETAILS SHALL BE THE FRICTION TYPE AND HERE SHALL BE NO PAINT, OIL, LAQUER, OR GALVANIZING BETWEEN THE CONTACT SURFACES. HIGH
- . HIGH STRENGTH BOLTS SHALL HAVE LOAD INDICATOR WASHERS TO SERVE AS A $\,$ DIRECT TENSION INDICATOR INSTALLATION FOR HIGH STRENGTH BOLTS SHALL REQUIRE INSPECTION BY A DEPUTY
- 3. ANCHOR RODS SHALL BE ASTM F-1554 GRD. 55 KSI U.N.O. ALL ANCHOR RODS SHALL BE. HEADED RODS.ANCHOR ROD WASHER SHALL BE ASTM A436. NUTS SHALL BE ASTM A563. 9. PIPE COLUMNS SHALL CONFORM TO ASTM A-53 GRADE B.
- 10. STEEL TUBE SHAPED MEMBERS SHALL CONFORM TO ASTM A-501 OR OR A-500 GRADE B. I1. WHERE FINISH IS ATTACHED TO STRUCTURAL STEEL, PROVIDE HOLES FOR 1/2" WELDED STUDS AT 4 FEET O.C. FOR THE ATTACHMENT OF NAILERS. SEE ARCHITECTURAL DRAWINGS FOR FINISHES.
- 12. OPEN WEB JOISTS SHALL COMPLY WITH THE STANDARDS OR "THE STEEL JOIST INSTITUTE". 13. STEEL STUDS, JOIST, TRACKS & BRIDGING: ASTM A-570 GRADE "E" Fy = 50 KSI 12, 14 & 16 GA. ASTM A-570 GRADE "C" Fy = 33 KSI 18 & 20 GA.

AUTHORITY USING ARC PROCESS WITH E70XX ELECTRODES.

14. SPECIAL INSPECTION OF HIGH-STRENGTH A325 AND A490 BOLTS SHALL BE IN ACCORDANCE WITH APPROVED NATIONALLY RECOGNIZED STANDARDS AND REQUIREMENT OF SECTION 1701 15. SHOP DRAWINGS SHALL BE PROVIDED TO ENGINEER OR ARCHITECT OF RECORD FOR REVIEW PRIOR

TO FABRICATION.

- 1. WELDING SHALL BE DONE BY THE ELECTRIC SHIELDED ARC PROCESS W/E70-XX ELECTRODES AND SHALL COMPLY WITH A.W.S. SPECIFICATIONS FOR WELDING AND FABRICATION. 2. WELDING SHALL BE PERFORMED BY CERTIFIED WELDERS WHO ARE APPROVED BY THE LOCAL
- 3. ALL FIELD WELDS SHALL HAVE CONTINUOUS INSPECTION PER CBC (1701) UNLESS OTHERWISE NOTED. 4. ALL BUTT WELDS SHALL BE FULL PENETRATION U.N.O.
- 5. A CERTIFICATE OF FABRICATION FROM THE SHOP PERFORMING WELDING OR A REPORT FROM THE SPECIAL INSPECTOR MUST BE FURNISHED TO THE JOB INSPECTOR PRIOR TO FRAMING APPROVAL 6. WELDED, FULLY RESTRAINED CONNECTION BETWEEN MEMBERS OF ORDINARY MOMENT FRAMES OR

SPECIAL MOMENT-RESISTING FRAMES SHALL HAVE SPECIAL CONTINUOUS INSPECTION AND

CONNECTION TESTED BY NONDESTRUCTIVE METHODS PER SECTION 1703.

7. FIELD WELDING OF REINFOCING STEEL SHALL BE DONE BY WELDERS SPECIFICALLY CERTIFIED FOR REINFORCING STEEL WELDING .BEFORE WELDING, THE "CARBON EQUIVALENT" (CE) OF STEEL SHALL BE DETERMINED. IF THE (CE) OF STEEL IS MORE THAN 0.75%, THEY SHALL NOT BE WELDED.

01 PLYWOOD DIAPHRAGM

1. FRAMING SHALL COMPLY WITH CHAPTER 23 OF THE 2010 CBC

FRAMING-GENERAL

- 1. USE SIMPSON U-HANGERS ON ALL JOIST/BEAM/BEAM CONNECTIONS UNLESS NOTED ON PLANS. 2. ALL POSTS SHALL HAVE SIMPSON "PC" CONNECTORS AT TOP AND SIMPSON "BC" OR "BCO" CONNECTORS AT
- 3. ALL CONNECTING HARDWARE, JOIST HANGERS, TIE STRAPS, ETC., SHALL BE SIMPSON "STRONG TIE" UNLESS 4. FRAMING @ CHIMNEY ENCLOSURE SHALL BE 2x6 STUDS BALLOON FRAMED W/APPROVED STRAPS TO ROOF AND

FRAMING - WALL

- I. SIZE, SPACING & HEIGHT LIMITS FOR WOOD STUDS ARE AS FOLLOWS (UNLESS OTHERWISE NOTED ON
- 2X4 @ 16" OC (BEARING WALL) SUPPORTING A MAXIMUM OF ONE FLOOR AND ONE ROOF SHALL HAVE A MAXIMUM HEIGHT OF 10 FEE 2X4 @ 16" OC (NON-BEARING WALL) SHALL HAVE A MAXIMUM HEIGHT OF 14 FEET 2X6 @ 16" OC (BEARING WALL) SUPPORTING A MAXIMUM OF TWO FLOORS AND A ROOF SHALL HAVE A MAXIMUM
- 2X6 @ 16" OC (NON-BEARING WALL) MAXIMUM HEIGHT IS 20 FEET 2 RAKE WALLS ADJACENT TO SLOPED CEILINGS SHALL BE BALLOON FRAMED. DOUBLE TOP PLATES SHALL
- ALWAYS BE SUPPORTED BY A ROOF OR CEILING DIAPHRAGM 3. SHEAR WALL PANELS MUST BE CONTINUOUS TO THE TOP PLATE AT ROOF FRAMING. SHEATHING SHALL HAVE ALL EDGES BLOCKED & THE APPROPRIATE SHEAR TRANSFER THRU CEILING OR SOFFIT FRAMING.
- 4. BORING AND NOTCHING OF WALL STUDS SHALL BE PER CBC (2308.9) NOTCHING MAXIMUM: 25% OF WIDTH OF STUDS ON BEARING WALLS
- 40% OF WIDTH OF STUDS ON NON-BEARING WALLS BORING MAXIMI IM: 40% OF WIDTH OF STUDS ON BEARING WALLS 60% OF WIDTH OF STUDS ON NON-BEARING WALLS
- NOTE: A MIN. 5/8" CLEARANCE FROM EDGE OF STUD TO HOLE SHALL BE PROVIDED. 5. DOUBLE 2X TOP PLATE SHALL BE LAPPED 48" AT ALL SPLICES AND SHALL OVERLAP AT CORNERS. 6. WALL BRACING SHALL BE PROVIDED PER CBC (2308.9.3)
- 7. HARDY FRAMES INSTALLATION PER MFR. SPECIFICATION (ICC ESR-2089)
- 8. STRONG WALL INSTALLATION PER MFR. SPECIFICATION (ICC-ESR-1267)

- I. FLOOR SHEATHING (MIN) 5/8" STRUCTURAL I T & G PLYWOOD PANEL INDEX NO. 32/16 WITH EXTERIOR GLUE. USE 10d COMMON NAILS AT 6" OC AT ALL EDGES, BOUNDARIES, AND 10" O.C. FIELD. NO BLOCKING IS REQUIRED
- UNLESS NOTED ON PLAN. ALL EDGES BLOCKED AT DECKS. 2. PROVIDE DOUBLE FLOOR JOISTS UNDER ALL PARALLEL NON- BEARING PARTITIONS.
- 3. PROVIDE CONTINUOUS BLOCKING BETWEEN FLOOR JOISTS UNDER BEARING WALLS WHICH ARE 4. FRAMING AROUND OPENINGS: TRIMMER AND HEADER JOISTS SHALL BE DOUBLED AND SUPPORTED BY HANGERS PER (CBC 2320.12.5).

. ROOF SHEATHING (MIN) 15/32" STRUC. I PLYWOOD SHEATHING PANEL INDEX NO. 32/16 WITH EXTERIOR GLUE. USE 8d COMMON NAILS AT 6" OC AT ALL EDGES, BOUNDARIES, AND 12" OC FIELD. NO BLOCKING IS REQUIRED UNLESS NOTED ON PLAN

2. FRAMING AROUND OPENINGS: TRIMMER AND HEADER JOISTS SHALL BE DOUBLED AND SUPPORTED BY

- HANGERS PER CODE FRAMING - CEILING (PER TABLE 2308.10.2)
- 1. CEILING JOISTS SHALL BE 2X6 @ 16" O.C. (MAX SPAN= 17'-8") 2. CEILING JOISTS SHALL BE 2X8 @ 16" O.C. (MAX SPAN= 23'-0")
- FRAMING JOISTS/RAFTERS 1. BORING AND NOTCHING OF JOISTS SHALL BE AS FOLLOWS: (CBC 2308.10) 2010 EDITION BORING- MAX DIA OF HOLE SHALL NOT EXCEED 1/3 OF DRESSED DEPTH OF JOIST WITH A MINIMUM EDGE CLEARANCE OF TWO INCHES.
- SHALL NOT EXCEED 1/6 OF THE JOIST DEPTH. 2. WHERE THREE OR MORE (MULTI JOISTS) ARE USED. THE JOISTS SHALL BE BOLTED TOGETHER WITH 1/2" DIA. MACHINE BOLTS W/ WASHERS AT 24" OC STAGGERED. BOLTS SHALL BE RETIGHTENED PRIOR TO

THE CENTER THIRD OF THE JOIST SPAN. MAX NOTCH IN TOP OR BOTTTOM OF THE JOIST

NOTCHING- MAX NOTCH AT ENDS SHALL NOT EXCEED 1/4 OF DEPTH. NO NOTCHING IS ALLOWED IN

3. JOISTS/RAFTERS SHALL LAP AT SPLICES A MIN. OF 4 INCHES WITH 3-16d NAILS OR USE SIMPSON ST 2115 @

4. CROSS BRIDGING OR 2X BLDG. SHALL BE PROVIDED @ 8'-0" O.C. MAX. FOR ALL JOISTS AND RAFTERS

- MORE THAN 8" IN DEPTH. 5. 2X SOLID BLOCKING SHALL BE PLACED BETWEEN JOISTS OR RAFTERS AT ALL SUPPORTS.
- 2. POSITIVE DRAINAGE AWAY FROM STRUCTURES SHALL BE AS FOLLOWS: 2% MIN. TO 21% MAX SWALES TO BE 3 FEET MIN. AWAY FROM STRUCTURES.

1. MINIMUM GRADIENTS ARE AS FOLLOWS: EARTH= 2%, PAVING= .5%

02 NAILING SCHEDULE

- 1. ALL BOLTS BEARING ON WOOD SHALL HAVE WASHERS UNDER HEAD OR NUT. SEE S.W. SCHEDULE. 2. ALL BOLTS SHALL BE RETIGHTENED, PRIOR TO APPLICATION OF PLYWOOD, PLASTER, ETC. 3. HOLES FOR BOLTS SHALL BE BORED 1/32" TO 1/16" LARGER THAN NOMINAL BOLT DIAMETER
- 4. FASTENERS IN PRESSURE-TREATED AND FIRE-RETARDANT, TREATED WOOD SHALL BE OF HOT-DIPPED.ZINC-COATED GALVANIZED STEEL.STAINLESS STEEL.SILICON BRONZE OR COPPER.
- 1. ALL LUMBER SHALL BE DOUGLAS FIR-LARCH OR THE FOLLOWING GRADES UNLESS OTHERWISE NOTED (MAX MOISTURE CONTENT SHALL NOT EXCEED 19% & GRADED IN ACCORDANCE WITH THE
- (WEST COAST LUMBERMAN'S ASSOCIATION.) REPETITIVE USE MEMBERS
- STUDS & PLATES NO. 2 JOISTS & RAFTERS: 2X4 TO 4X4 INCLUSIVE
- 2X6 TO 3X16 INCLUSIVE SINGLE USE MEMBERS
- 6X OR LARGER No. 1 POSTS & MULLION: 4X4 & SMALLER 6X6 & LARGER
- BLOCKING, FURRING, ETC. No. 3 **DECKING & SHEATHING** 2X, 3X, 4X CONST. GRADE
- 2. ALL WOOD BEARING ON CONCRETE OR MASONRY IF LESS THAN 4 FEET FROM GRADE SHALL BE PRESSURE TREATED DOUG. FIR. GLUED-LAMINATED WOOD BEAMS SHALL BE DOUGLAS FIR COMB. 24F-V4 (*) DF/DF (Fb=2400 PSI, Fv=165 PSI, E=180,000 PSI) INDUSTRIAL APPEARANCE WITH EXTERIOR GLUE UNLESS OTHERWISE NOTED ON PLANS, A CERTIFICATE OF INSPECTION FOR EACH GLU-LAM BEAM FROM AN APPROVED TESTING
- AGENCY TO BE SUBMITTED AND APPROVED BY THE BUILDING DEPT. PRIOR TO ERECTION. (*) USE V8 FOR CANT. BEAMS AND V4 FOR SIMPLE SPANS BEAMS]
- 4. SHOP DRAWING SHALL BE PROVIDED TO ENGINEER OR ARCHITECT OF RECORD FOR REVIEW PRIOR 5. ALL STRUCTURAL PLYWOOD SHALL BE IN ACCORDANCE WITH (PS 1-95)

6. PARALLAM PSL PER TRUS JOIST MACMILLAN (ICC ESR-1387) (Fb= 2900 PSI, Fv=290 PSI, E= 2,000,000 PSI)

7. TJI JOISTS INSTALLATION PER MANUFACTURE SPECIFICATION (ICC ES ESR-1153 AND ICC ES ESR-1387)

SPECIAL INSPECTION (PER CBC SECTION 1704,1706 & 1707) . SPECIAL INSPECTION BY A REGISTERED DEPUTY BUILDING INSPECTOR, APPROVED BY THE ARCHITECT AND THE CHECKING AGENCY, SHALL BE REQUIRED FOR THE FOLLOWING TYPES OF WORK. SEE PROJECT SPECIFICATIONS FOR SPECIFIC REQUIREMENTS. SPECIAL INSPECTIONS SHALL NOT BE REQUIRED WHEN THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND

APPROVED BY THE BUILDING OFFICIAL TO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION.

ITEMS REQUIRE SPECIAL INSPECTION AS MARKED:

- VERIFICATION & INSPECTION CONTINUOUS PERIODIC STRUCTURAL EPOXY BOLTING 2a: FIELD WELDING OF MOMENT RESISTING STEEL FRAMES. ___ 2b: STRUCTURAL STEEL OR REINFORCING 2c: STEEL DECKING. 2d: SHEAR CONNECTORE PLACEMENT OF REINFORCING STEEL IN CMU WALL. 4. HIGH STRENGTH BOLTING 5. EXPANSION TYPE ANCHOR BOLTS. 6. HIGH STRENGTH BOLTING CONCRETE WHERE CONCRETE STRENGTH OF 3000 PSI OR GREATER IS SPECIFIED. DIAPHRAGM CONNECTION TO STEEL SUPPORT MEMBERS D. WOOD SHEAR WALLS AND WOOD DIAPHRAGMS NAILING. . WOOD STRUCTURAL PANEL SHEATHING . NOMINAL SIZE OF FRAMING MEMBERS AT PANEL EDGES . NAIL OR STAPLE DIAMETER AND LENGTH. COMPACTED FIL 5. FOUNDATION -ANCHOR BOLT AND HOLD DOWN. 6. INSPECTION OF LATERAL FORCE RESISTING ELEMENTS.
- "CONTRACTOR RESPONSIBILITY: EACH CONTRACTOR OR SUB-CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF THE WIND AND/OR SEISMIC RESISTING SYSTEM THAT IS LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK REQUIRING SPECIAL INSPECTION. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL CONTAIN THE FOLLOWING:
- 1) ACKNOWLEDGMENT OF AWARENESS OF THE SPECIAL REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS 2) ACKNOWLEDGMENT THAT CONTROL WILL BE EXERCISED TO OBTAIN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE BUILDING OFFICIAL;
 3) PROCEDURES FOR EXERCISING CONTROL WITHIN THE CONTRACTOR'S ORGANIZATION THE METHOD AND FREQUENCY OF REPORTING AND THE DISTRIBUTION OF THE REPORTS; B) PROCEDURES FOR EXERCISING CONTROL WITHIN THE CONTRACTOR'S ORGANIZATION THE METHOD AND FREQUENCY OF REPORTING AND THE DISTRIBUTION OF THE REPORTS;

4) IDENTIFICATION AND QUALIFICATIONS OF THE PERSON(S) EXERCISING SUCH CONTROL

AND THEIR POSITION(S) IN THE ORGANIZATION."

CEILING JOIST COLUMN CONCRETE CONTINUOUS DOUGLAS FIR DIAMETER DITTO EXISTING FACH WAY EXPANSION JOIN EDGE NAIL EQUAL FLOOR BEAM FINISH GRADE FLOOR JOIST FACE OF CONCRETE FACE OF MASONRY FACE OF STUDS FULL PENETRATION GALVANIZED GLUE-LAMINATED BEAM GRADE BEAM GYPSUM WALLBOARD HEADER

- FAMII EMOI 6th AV
- LAMINATED VENEER LUMBER MICRO=LAM BEAM NATURAL GRADE PARALLAM PSL BEAM PRESSURE TREATED THREADED ROD
- THESE DRAWINGS AND SPECIFICATIONS AS INSTRUMENT OF SERVICE ARE PROVIDED COMBINED WITH OTHER PLANS AND SPECIFICATIONS TO OBTAIN BUILDING NOT INTENDED TO, NOR DO THEY, DETAIL ALL CONDITIONS, IDENTIFY ALL MATERIALS THE BUILDER ASSUMES RESPONSIBILITY TO SELECT ALL MATERIAL AND ALL TO PROVIDE ENOUGH INFORMATION ABOVE AND BEYOND THESE DRAWINGS, TO COMPLETE THE PROJECT IN CONFORMANCE WITH ALL GOVERNING AGENCIES.

ABBREVIATIONS

ANCHOR BOLT

BOUNDARY NAIL

CONTINUOUS FOOTING

BOTH WAYS

CEILING

DOUBLE

FOOTING

HANGER HORIZONTAL

KING POST

LIGHT WEIGHT

MACHINE BOLT

ON CENTER

POST ABOVE

RIDGE BEAM

REQUIRED

ROOF

TYPICAL

REINFORCING

ROOF RAFTER

PI YWOOD

LENGTH

ABOVE REINF. BAR

BOARD

BELOW

BI OCKING

A.B. ABV. BAR BD BLKG. BLW.

B.W. CONT.FTG.

FTG

GRD. BM.

GWB

HNGR. HORIZ.

LT. WT.

REQ'D.

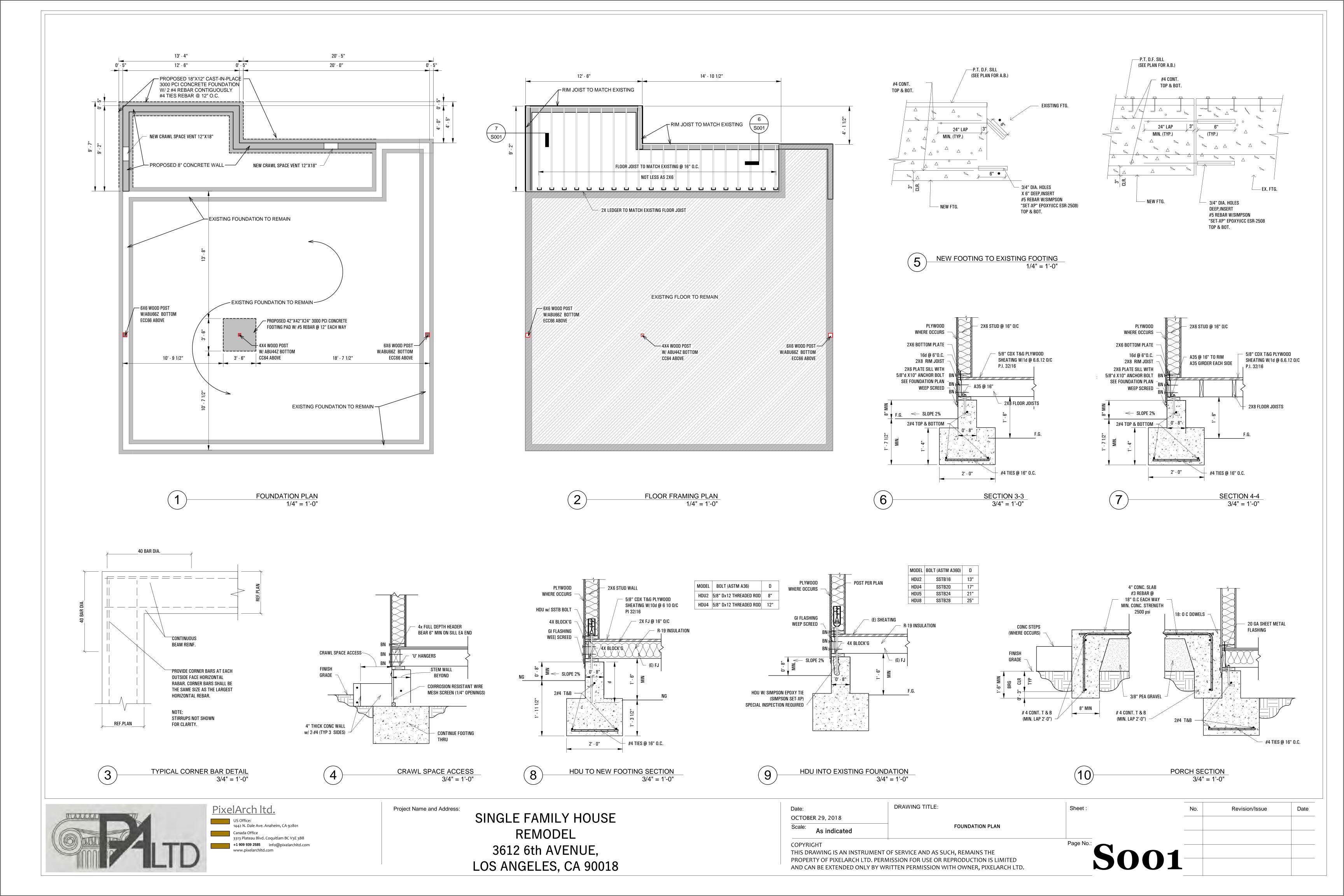
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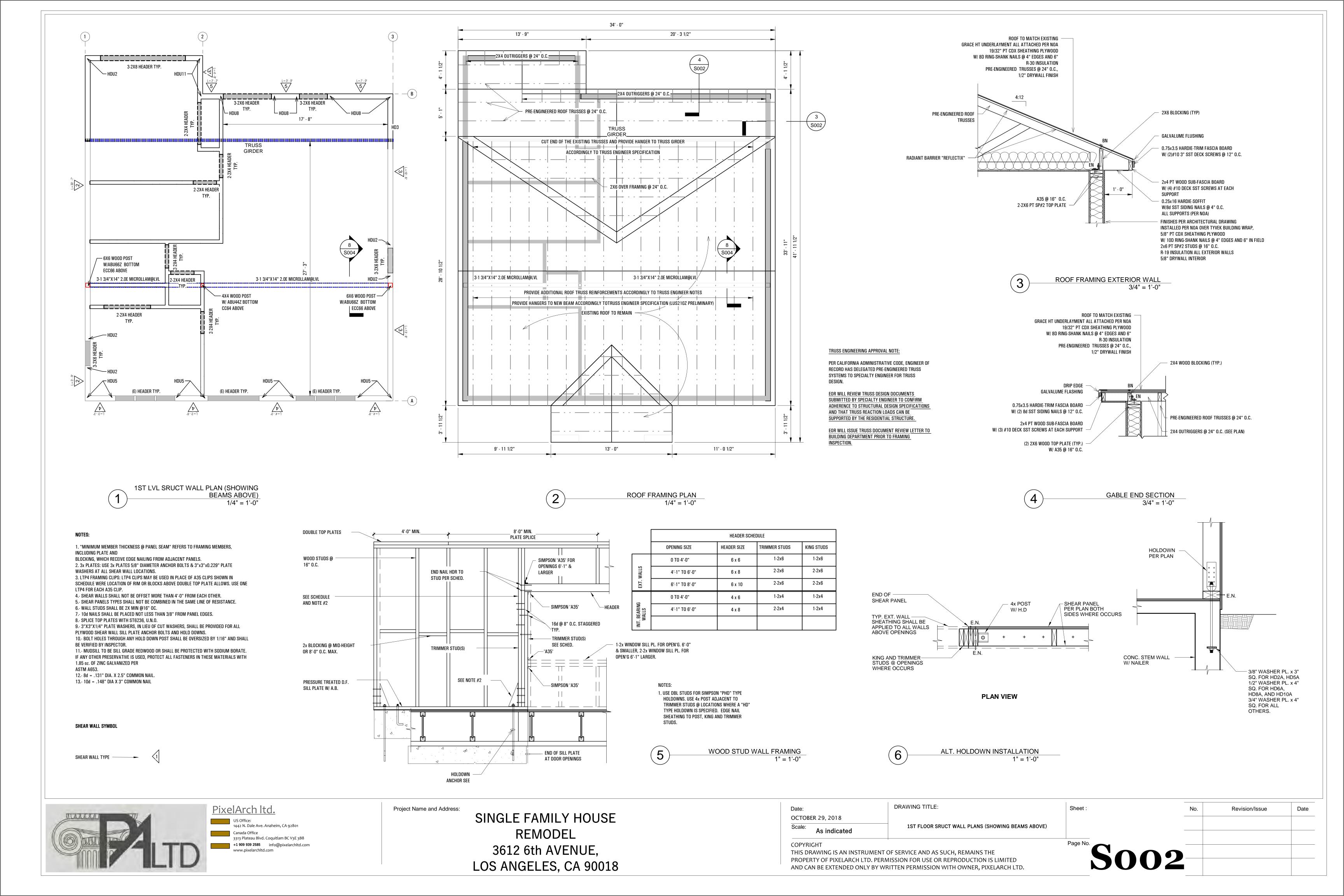
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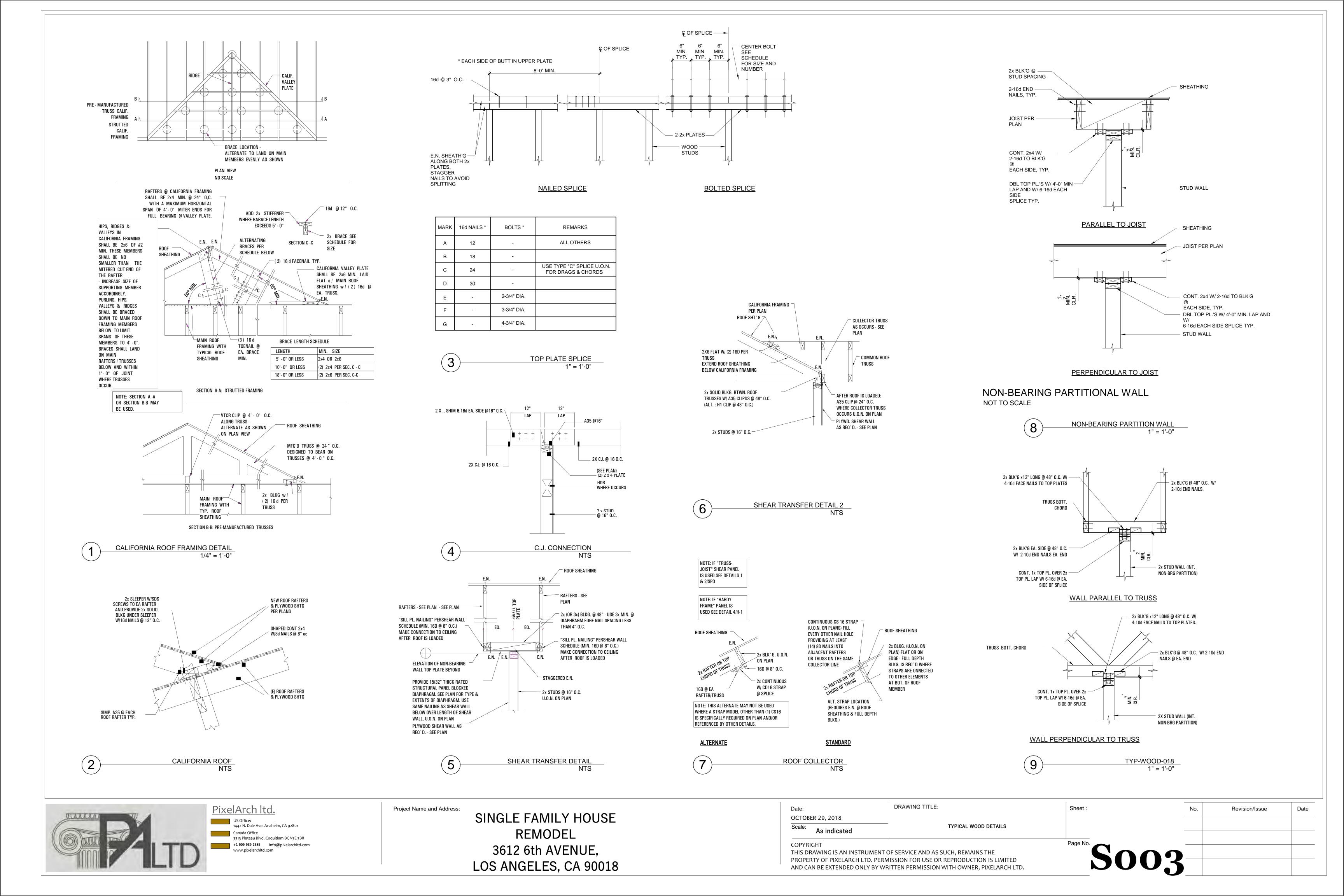
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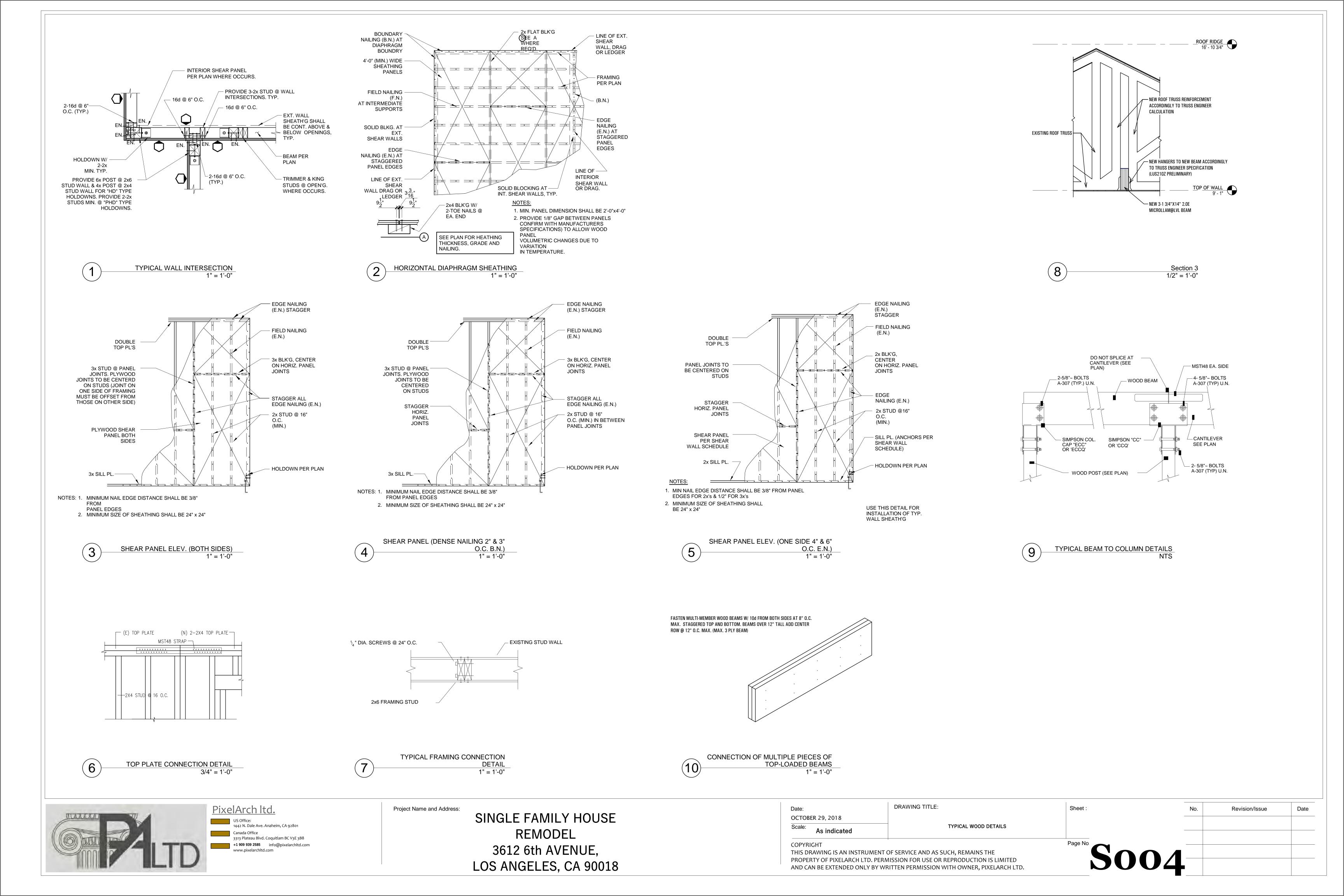
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GENERAL NOTES









MEP DESIGNS

SINGLE FAMILY HOUSE REMODEL

3612 6th AVENUE, LOS ANGLES, CA 90018









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US Office:
1442N. Dale Ave. Anaheim, CA 92801

Canada Office
3313Plateau Blvd. Coquitlam BC V3E 3B8

+1 909 939 2585 info@pixelarchltd.com www.pixelarchltd.com Project Name and Address:

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NEC considerations:

- 210.12Arc-Fault Circuit-Interrupter Protection. Arc fault circuit—interrupter protection shall be provided as required in (210.12A) and (B). The arc-fault circuit interrupter shall be installed in a readily accessible location
- (A) Dwelling Units. All -120volt, single phase -15, and -20ampere branch circuits supplying outlets or devices installed in dwelling unit kitchens, family rooms, dining rooms, living rooms, parlors, libraries, dens, bedrooms, sunrooms, recreation rooms, closets, hallways, laundry areas, or similar rooms or areas shall be protected as described by), 3), (2), (1(or)4(
-)1(A listed combination type arc-fault circuit interrupter, installed to provide protection of the entire branch circuit.
-)2(A listed outlet branch circuit type arc-fault circuit interrupter installed at the first outlet on the branch circuit where all of the following conditions are met:
- a. The branch circuit over current protection device shall be a listed circuit breaker having an instantaneous trip not exceeding 300 amperes
- The branch circuit wiring shall be continuous from the branch circuit overcurrent device to the outlet branch circuit arc-fault circuit interrupter
- c. The maximum length of the branch circuit wiring from the branch circuit overcurrent device to the first outlet shall not exceed 15.2m 50(ft) for a 14AWG or 21.3m 70(ft) for a 12AWG conductor— d. The first outlet box in the branch circuit shall be identified .
- (1) A listed outlet branch circuit type arc-fault circuit interrupter installed at the first outlet on the branch circuit where the portion of the branch circuit between the branch-circuit overcurrent device and the first outlet is installed using RMC, IMC, EMT, Type MC, or steel armored Type AC cables meeting the requirements of
- 250.118and using metal outlet and junction boxes.
- (2) A listed outlet branch circuit type arc-fault circuit interrupter installed at the first outlet on the branch circuit where the portion of the branch circuit between the branch-circuit overcurrent device and the first outlet is installed using a listed metal or nonmetallic conduit or tubing encased in not less than 50mm 2(in.) of concrete.

NEC considerations:

- 406.12 Tamper Resistant Receptacles.
- (A) Dwelling Units. In all areas specified in , 210.52all non-locking-type -125volt-15, and -20ampere receptacles shallbe listed tamper-resistant receptacles.
- Guest Rooms and Guest Suites of Hotels and Motels. All non-locking-type -125volt- 15, and -20ampere receptacles located in guest rooms and guest suites of hotels and motels shall be listed tamper—resistant receptacles.
- Child Care Facilities. In all child care facilities, all non-locking-type -125volt-15, and -20ampere receptacles shall be listed tamper-resistant receptacles.
- Exception to (A), (B), and (C): Receptacles in the following locations shall not be required to be tamper-resistant
- Receptacles located more than 1.7m extstyle 1.25(ft) above the floor.
- Receptacles that are part of a luminaire or appliance.
- A single receptacle or a duplex receptacle for two appliances located within dedicated space for each appliance that, in normal use, is not easily moved from one place to another and that is cord-and-plug connected in accordance with (400.7A), (6)(A), 7)(or (A) 4). (8)(Non-grounding receptacles used for replacements as permitted in (406.4D) (2) (a).

	Electrical plans
Sheet No.	Issue
E00	Legends, symbols and NEC codes
E01	Appliances and Wiring
E02	Lighting circuits
E03	Photometric Data
E04	Photometric studies and FC levels
E05	MDB and DB panel schematics
E06	Grounding service and protections
E07	Data and CATV
E08	Fire Alarm
E09	
E10	
E11	

Note:

- 1. All receptacles are TAMPER RESISTANT RECEPTACLES
- 2. The main disconnector in DP is AFCI
- 3. receptacles in bathrooms are all GFI type

		. —
SYMBOL	DEFINITION	NOTES
	125V DUTLET	20 AMP SINGLE POLE
	110∨ DUTLET	20 AMP two POLE
GFI	110∨ OUTLET	EQUIPED WITH GROUND FAULT INTERRUPTER (TAMPER RESISTANT TYPE)
	220V DUTLET	4 WIRE CONNECTION
WP GFI	EXTERIOR WATERPROOF OUTLET	GROUND FAULT INTERRUPTER
\$	SWITCH	
	WALL MDUNTED LIGHT	
\searrow	CEILING LIGHT	
	PANEL	
	METER	
•S'D'	SMOKE DETECTOR	
⊗s/co	CARBON MONOXIDE/ SMOKE DETECTOR COMBO	BATT. BACK-UP W/ HARDWIRE INTER-CONNECTED SHALL BE A DISTANO OF NOT LESS THEN A FROM WALL
€ EF	Exhaust fan JACK	
WH	WATER HEAT. DISCONNECT	IGNITION SOURCE TO BE ELEVATED MIN. 18" AFF.
X	Light mounted Fan	
TV.	TELEVISION JACK	

ELECTRICAL LEGEND

1 of 9

- * LOCATION OF TV JACKS & PHONE OUTLETS & FANS TO BE VERIFIED @ HOMEOWNER PRE-CONSTRUCTION MEETING.
- ' ALL RECEPTACLES IN ALL HABITABLE ROOMS TO BE ARC FAULT PROTECTED PER ELECTRICAL PROVISIONS OF FBCR 5TH EDITION (2011).

External water proof

BATHROOM EXHAUST FAN TO HAVE MIN. CAPACITY OF 50 CFM INTERMITTENT PER ELECTRICAL PROVISIONS OF SECTION M1507.3 FBCR 5TH EDITION (2011)

'LAUNDRY ROOM RECEPTACLE SHALL BE

- GROUND FAULT CIRCUIT-INTERRUPTER PROTECTION FOR PERSONNEL ON FEEDERS WIRING METHOD SHALL BE NON METALLIC CABLE PER ELECTRICAL PROVISIONS OF FBCR 5TH ED (2011).
- ' ALL RECEPTACLES TO BE TAMPER-RESISTANT TYPE
- *ALL WORK TO COMPLY WITH ELECTRICAL PROVISIONS OF THE FBCR 5TH ED (2011).

TITLE 24 NOTES

- 1. ALL HIGH EFFICACY LUMINAIRES SHALL BE SWITCHED SEPARATELY FROM LOW EFFICACY LUMINAIRES.
- 2. HIGH-EFFICACY LUMINAIRES SHALL CONSTITUTE MIN. 50% OF TOTAL WATTAGE IN KITCHEN LIGHTING. ADDITIONAL 50-WATTS OF LOW-EFFICACY LUMINAIRES ARE
- PERMITTED FOR DWELLING UNIT UNDER 2,500-SF AND ADDITIONAL 100-WATTS ALLOWED FOR OVER 2,500-SF.
- 3. ALL LOW-EFFICACY KITCHEN LUMINAIRES SHALL BE CONTROLLED BY CEC APPROVED VACANCY SENSOR OR DIMMER.
- 4. NO MORE THAN 20-WATTER PER LINEAR FOOT OF PERMANENTLY INSTALLED INTERNAL CABINET LIGHTING IS PROHIBITED.
- 5. ALL LOW-EFFICACY BUILDING MOUNTED EXTERIOR LUMINAIRES SHALL BE CONTROLLED BY PHOTOCELL & MOTION SENSOR.
- 6. ALL LOW-EFFICACY LUMINAIRES IN BATHROOMS MUST BE CONTROLLED BY CEC APPROVED VACANCY SENSOR OR TIMER.
- 7. ALL LOW EFFICACY LUMINAIRES LOCATED IN GARAGE, LAUNDRY ROOM, CLOSETS, AND UTILITY ROOMS SHALL BE CONTROLLED BY CEC APPROVED VACANCY SENSOR.
- 8. ALL LOW EFFICACY LUMINAIRES IN AREAS OTHER THAN THOSE LISTED ABOVE SHALL BE CONTROLLED BY DIMMERS OR CEC APPROVED VACANCY SENSOR.
- 9. ALL FIXTURES INSTALLED IN INSULATED CEILINGS MUST BE C-RATED & LABELED, AND OF AIR-TIGHT CONSTRUCTION BEARING AN ASTM E283 COMPLIANCE LABEL, AND
- SHALL BE SEALED WITH A GASKET OR CAULK BETWEEN THE HOUSING AND CEILING.
- 10. ALL EXHAUST FANS SHALL BE SWITCHES SEPARATELY FROM LUMINAIRES.
- 11. NO SWITCH SHALL BYPASS DIMMER OR CEC APPROVED VACANCY SENSOR.



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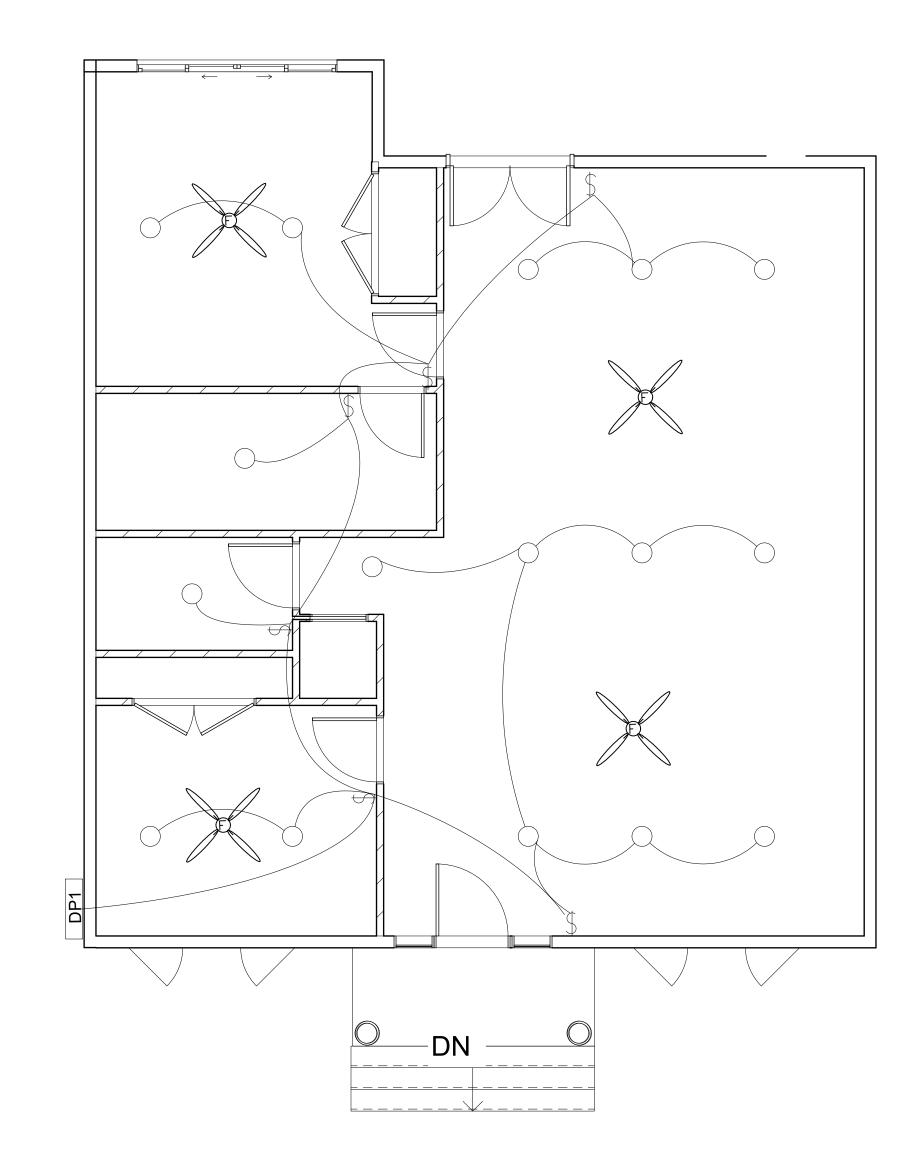
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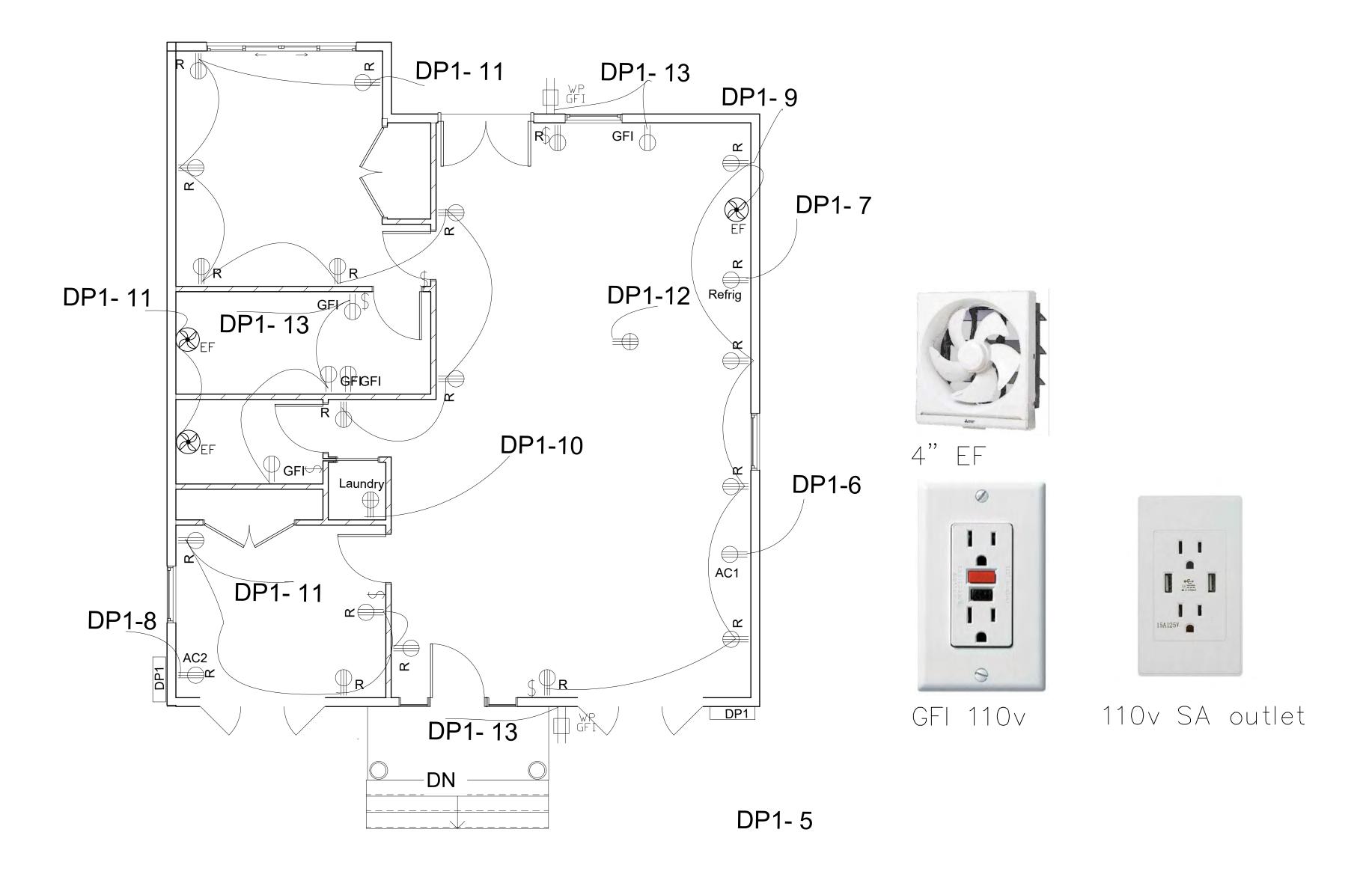
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Appliances Plan

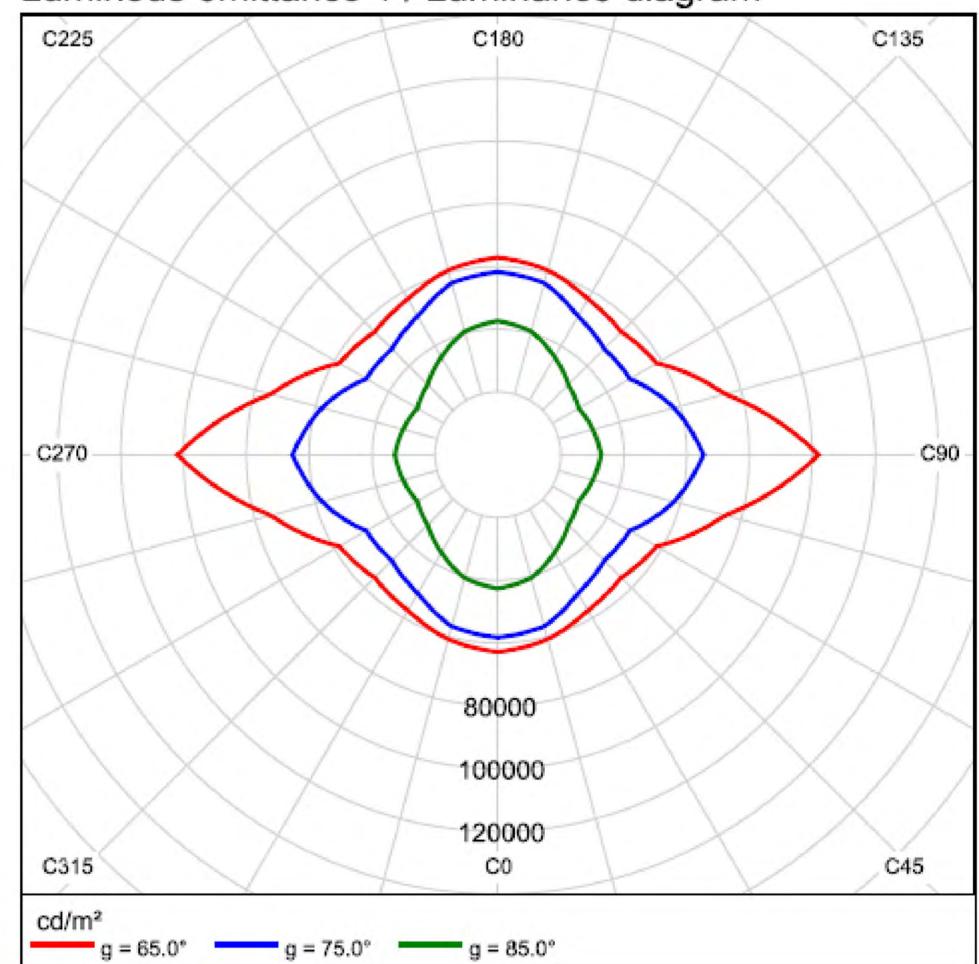
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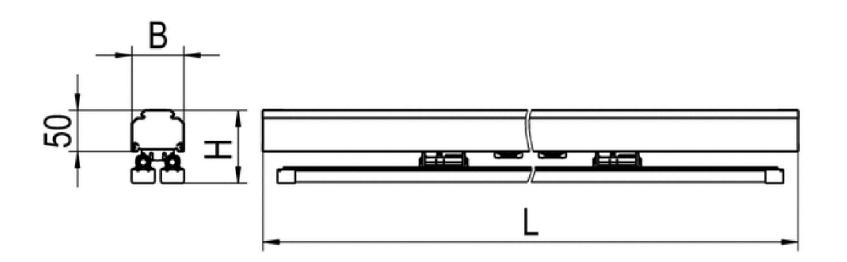
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Luminous emittance 1 / UGR diagram

Glare evaluation according to UGR											
ρ Ceilin	g	70	70	50	50	30	70	70	50	50	30
ρ Walls		50	30	50	30	30	50	30	50	30	30
ρ Floor		20	20	20	20	20	20	20	20	20	20
Room X	size Y	Viewi	ng dired to	ction at lamp ax	-	ngles	Vie		irection amp a		llel
2H	2H 3H 4H 6H 8H 12H	25.6 27.4 28.1 28.7 28.8 29.0	26.8 28.4 29.1 29.6 29.7 29.8	25.9 27.7 28.5 29.0 29.2 29.3	27.0 28.7 29.4 29.9 30.0 30.1	27.2 29.0 29.7 30.2 30.4 30.5	28.8 30.0 30.4 30.7 30.7 30.8	30.0 31.0 31.4 31.6 31.6 31.6	29.1 30.3 30.7 31.0 31.1 31.1	30.2 31.3 31.7 31.9 31.9 31.9	30.4 31.6 32.0 32.2 32.2 32.3
4H	6H 8H	26.3 28.2 29.1 29.7 30.0 30.1	27.3 29.0 29.8 30.4 30.6 30.7	26.6 28.6 29.5 30.2 30.4 30.6	27.5 29.4 30.2 30.8 31.0 31.1	27.8 29.7 30.5 31.2 31.4 31.5	28.9 30.3 30.8 31.2 31.3 31.4		31.8	30.2 31.4 31.9 32.2 32.3 32.3	30.5 31.8 32.3 32.6 32.7 32.8
8H	4H 6H 8H 12H	29.3 30.1 30.4 30.7	29.9 30.6 30.9 31.0	29.7 30.6 30.9 31.1	30.3 31.0 31.3 31.5	30.7 31.5 31.8 32.0	30.9 31.4 31.6 31.7	32.0	31.9	31.9 32.3 32.4 32.5	32.3 32.8 32.9 33.0
12H	4H 6H 8H	29.3 30.2 30.5	29.9 30.6 30.9	29.8 30.6 31.0	30.3 31.0 31.3	30.7 31.5 31.8	30.9 31.4 31.6		31.4 31.9 32.1	31.9 32.3 32.4	32.3 32.8 32.9

Luminous emittance 1 / Luminance diagram





Light output ratio: 86.27%

Lamp luminous flux: 10200 lm

Luminaire luminous flux: 8800 lm

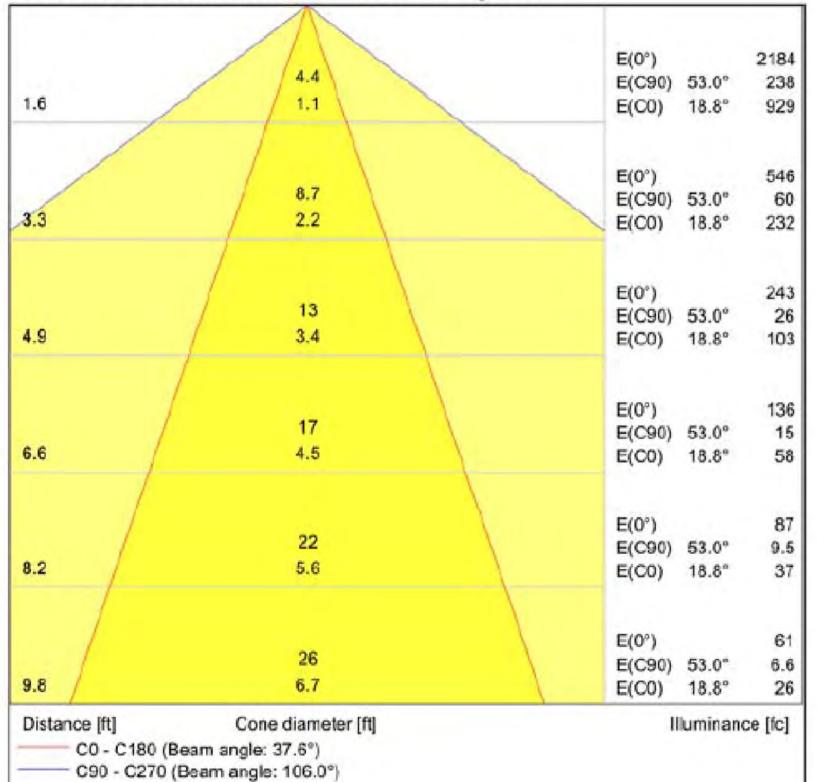
Power: 68.0 W

Luminous efficacy: 129.4 lm/W

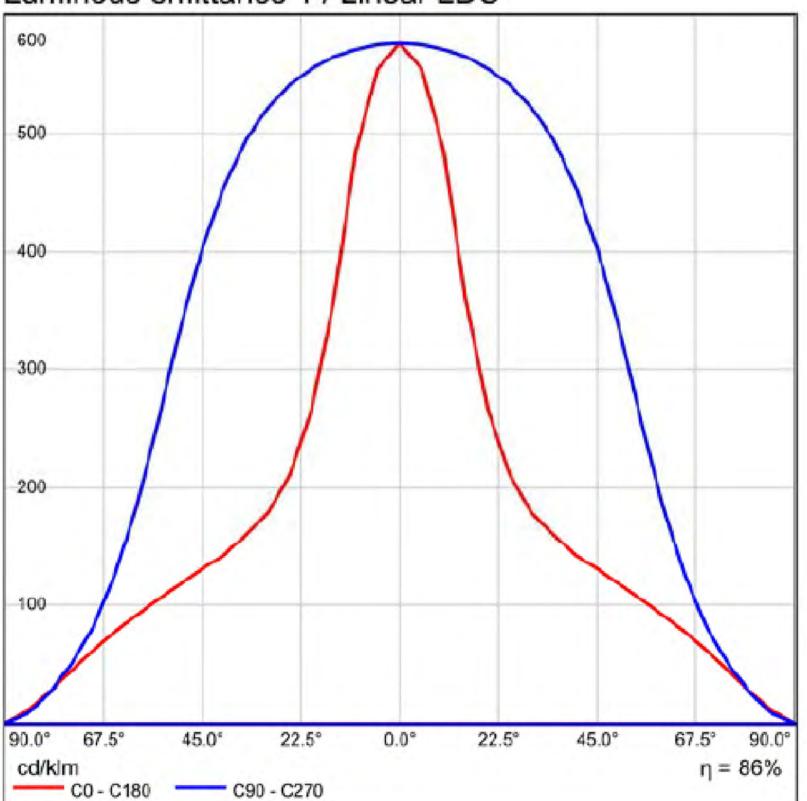
Colorimetric data

1xL-TUBE-TF2 60W GF: CCT 4000 K, CRI 80

Luminous emittance 1 / Cone diagram



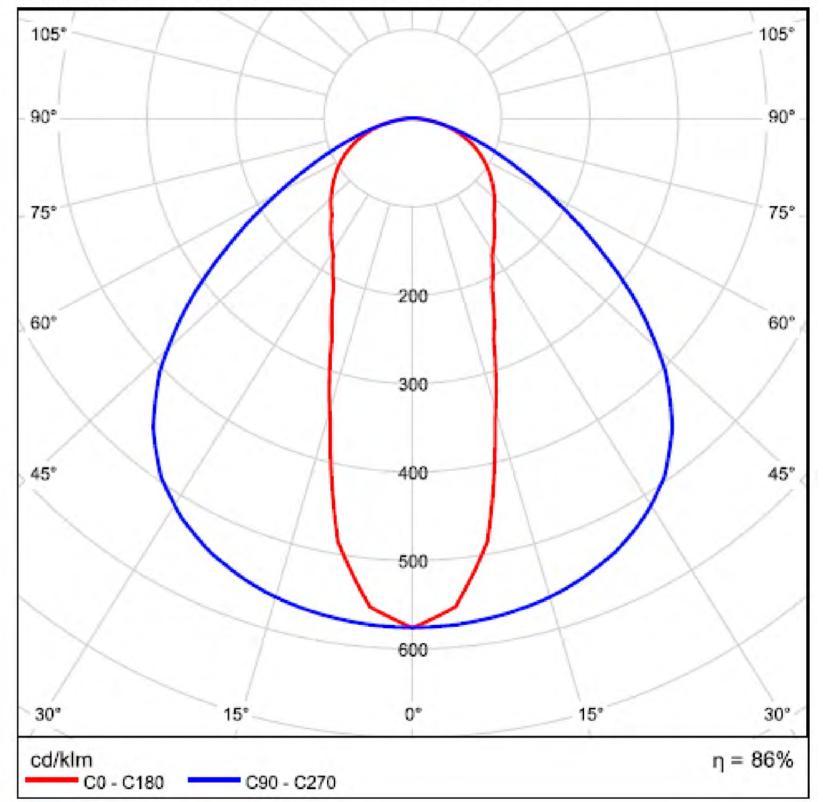




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Luminous emittance 1 / Polar LDC



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Lamps: 1xL-TUBE-TF2 66 W

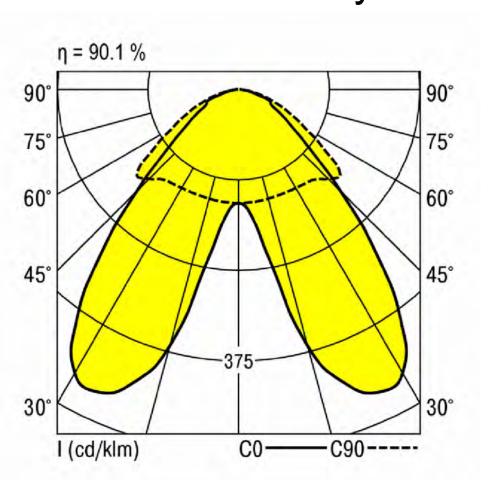
Lamp type: L-TUBE-TF2

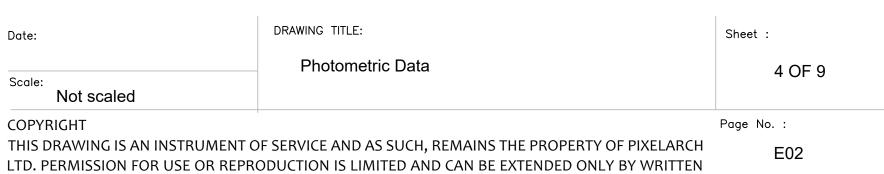
Number of lamps: 1
Wattage: 66 W
Luminous colour: 840
LED system values

Luminaire luminous flux: 9190 lm

Luminaire output: 68 W

Luminaire efficiency: 135 lm/W





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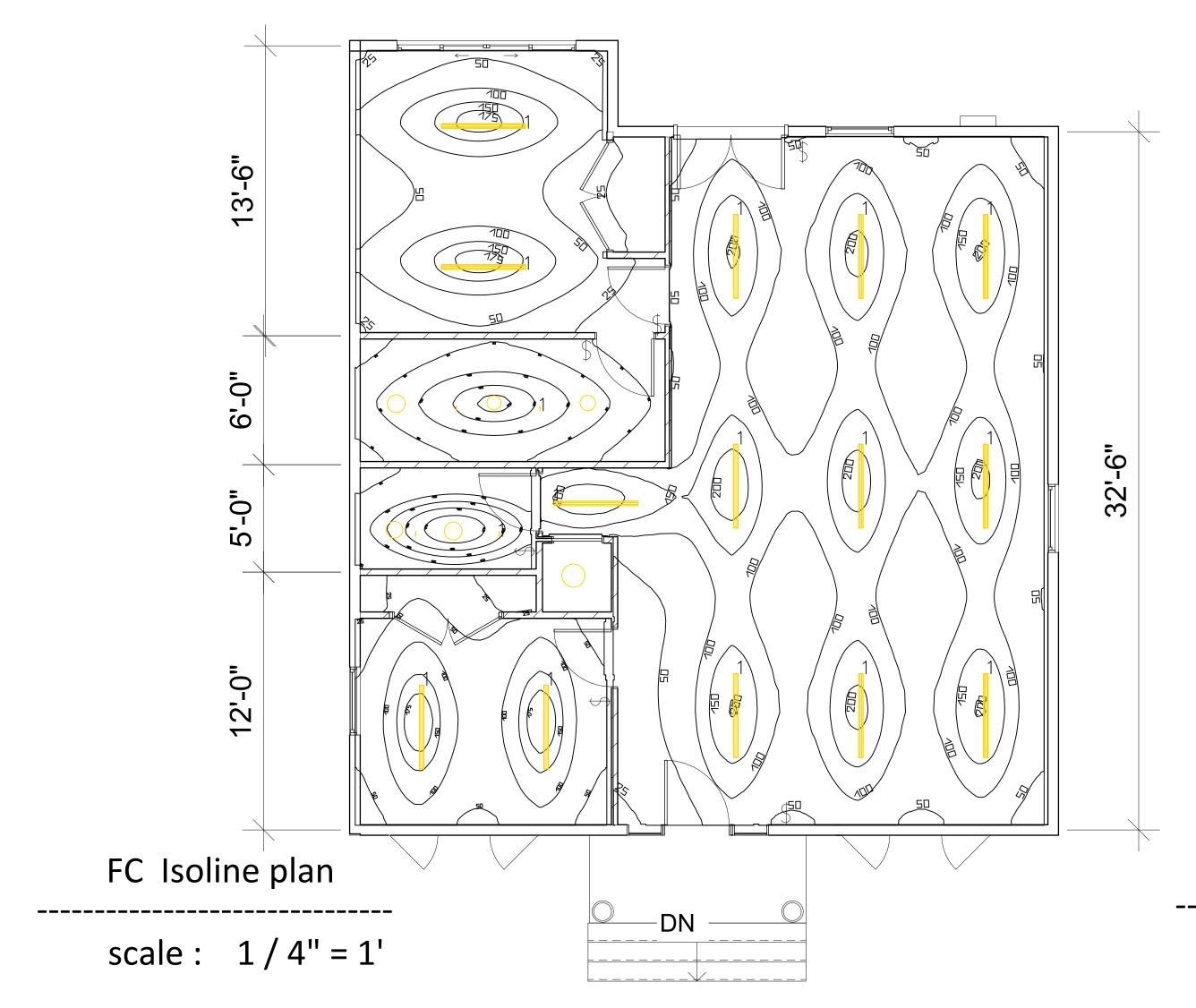
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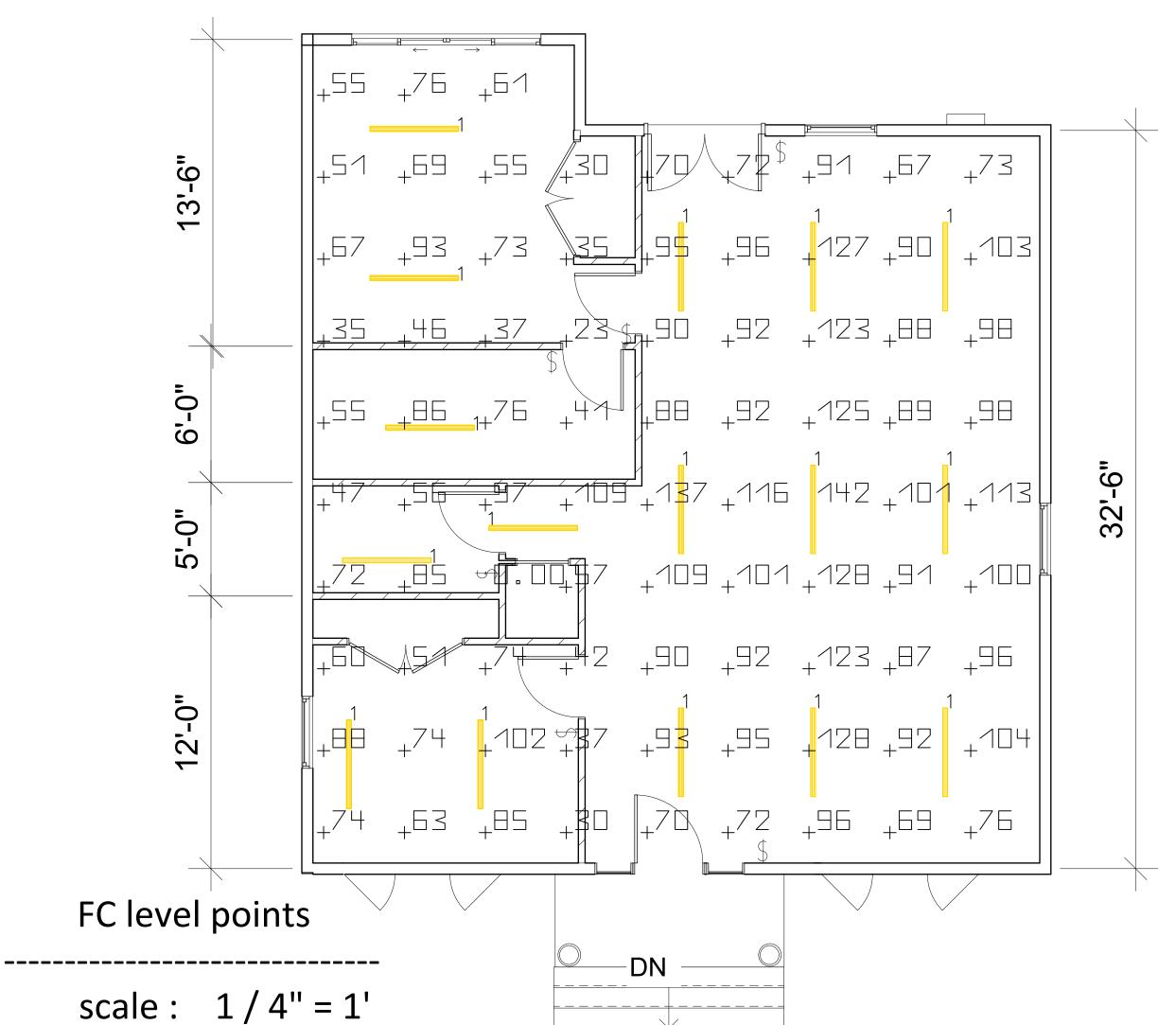
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E02



#	Name	Parameter	Min	Max	Average	Min/average	Min/max
1	Calculation surface	Perpendicular illuminance	0.00 fc	142 fc	80.4 fc	0.00	0.00
2	Workplane 1	Perpendicular illuminance (Adaptive)	11.2 fc	186 fc	68.2 fc	0.164	0.060
3	Workplane 2	Perpendicular illuminance (Adaptive)	14.5 fc	181 fc	64.5 fc	0.224	0.080
4	Workplane 3	Perpendicular illuminance (Adaptive)	38.3 fc	192 fc	106 fc	0.360	0.200
5	Workplane 4	Perpendicular illuminance (Adaptive)	6.90 fc	194 fc	85.8 fc	0.080	0.036
6	Workplane 5	Perpendicular illuminance (Adaptive)	22.7 fc	230 fc	107 fc	0.213	0.099
7	Workplane 6	Perpendicular illuminance (Adaptive)	0.00 fc	0.000 fc	0.00 fc	/	/



Luminaire parts list								
Index	Manufacturer	Luminaire type	Item number	Fitting	Luminous flux	Light loss factor	Connected load	Quantity
1	RIDI	VLG-TF254-5ND-1 020E840	0250177	1xL-TUBE-T F2 60W GF	10200 lm	0.80	68 W	16



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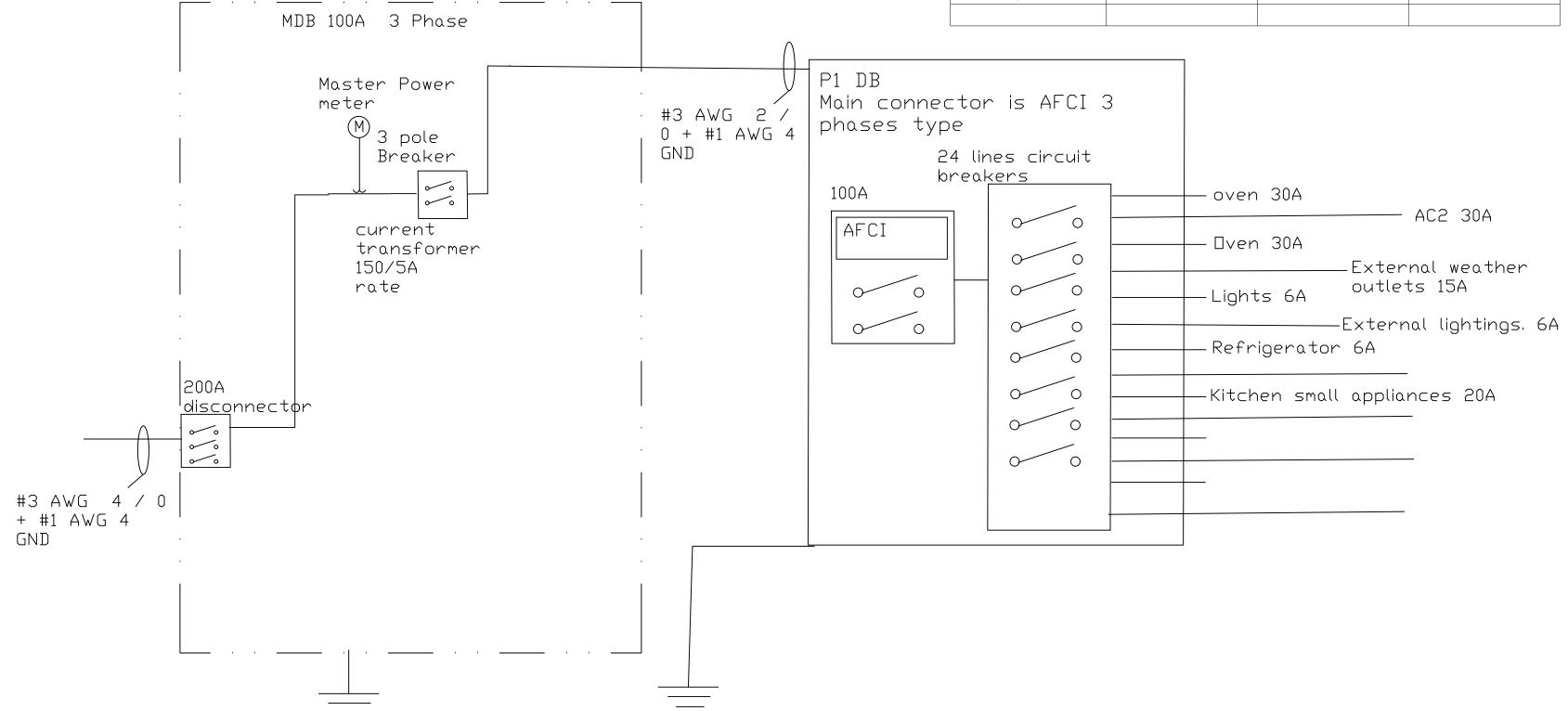
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	MAIN:	100A MCB	GR FL	_00R				1		VOI	LTAGE:	240/12	20	PHAS	SE:3	WIRI	≣: 4	MOUN	TING: SURFACE AIC:	22,000	
CKT	TRIP				LOAE	(KVA)					ASE	Τ) (KVA					TRIP	CK.
#	POLE	DESCRIPTION	LTG	REC	MTR	A/C	HTG	KIT I	MISCA	В	С	LTG	REC	MTR	A/C		KIT	MISC	DESCRIPTION	POLE	#
1	30/2	Oven					5.0									3.0			Water Heater	30/1	2
3	30/2	Overi																	- vvaler neater	30/1	4
5	15/1	Lightings	1.1												1.0				AC1	30/1	6
7	15/1	Refrigerator		0.8											1.0				AC2	30/1	8
9	20/1	Kitchen Appliances		1.8									0.5						laundry	20/1	10
11	20/1	Small Appliances		1.8									2.0						Dish washer	20/1	12
13	20/1	GFIs		0.9										1.2					Garbage dispose	15/1	14
15	20/1																				16
17 19	20/1 20/1																				18 20
	TING (K	VA): 1.1	1.1	5.3	0.0	0.0	5.0	0.0	0.0			0.0	2.5	1.2	2.0	3.0	0.0	0.0	CONNECTED LOAD (KVA): 20	
		ES (KVA): 7.8		1		1		<u> </u>											DEMAND LOAD (KVA):	20	
		ump Pumps (KVA 1.2						PHAS	SE A	14	1.5	1	20.8								
/C (KVA):	2.0					F	PHASI	ЕВ	5.	.6		46.3						CONNECTED LOAD (AMP	'S): 83	3.5
IEA ⁻	TING (K'	VA): 8.0					F	PHASE	E C				0.0						DEMAND LOAD (AMPS):	83	3.5
ITC	HEN (KV	VA): 0.0								K۱	VA	Α	MPS								
/IISC	ELLAN	EOUS (KVA): 0.0																	AMPACITY REQUIRED:	84	ŀ.7

BREAKERS PROTECTING MULTI-WIRE BRANCH CIRCUITS SHALL BE FIELD-EQUIPPED WITH A MANUALLY OPERATED HANDLE-TIE DEVICE TO ENSURE THAT ALL UNGROUNDED CONDUCTORS ARE SIMULTANEOUSLY DISCONNECTED PER NEC 240.15.

Size (kcmil)	Amperes	Size (kcmil)	A mperes
250	119	2500	376
500	168	3000	412
750	206	3250	429
1000	238	3500	445
1250	266	3750	461
1500	292	4000	476
1750	315	4250	491
2000	336	4500	505
2250	357	4750	519

	House load	schedule	
load	Power (KW.)	Qty	Description
Oven	5	1	kitchen stuff
lights	1.1	As mentioned in photometric plan	3W/sq.ft
refrigerator	0,8	1	Kitchen Stuff
Kitchen appliances	1.8	considered for small appliances	Kitchen stuff
Water heater	3	2	instant Water heater one installed in bath room and another one installed in the kitchen
A/C unit	1	2	for living room and bed room
Laundry	0,5	1	4 cu ft capacity
Dishwasher	2	1	
Garbage disposal	1,2	1	





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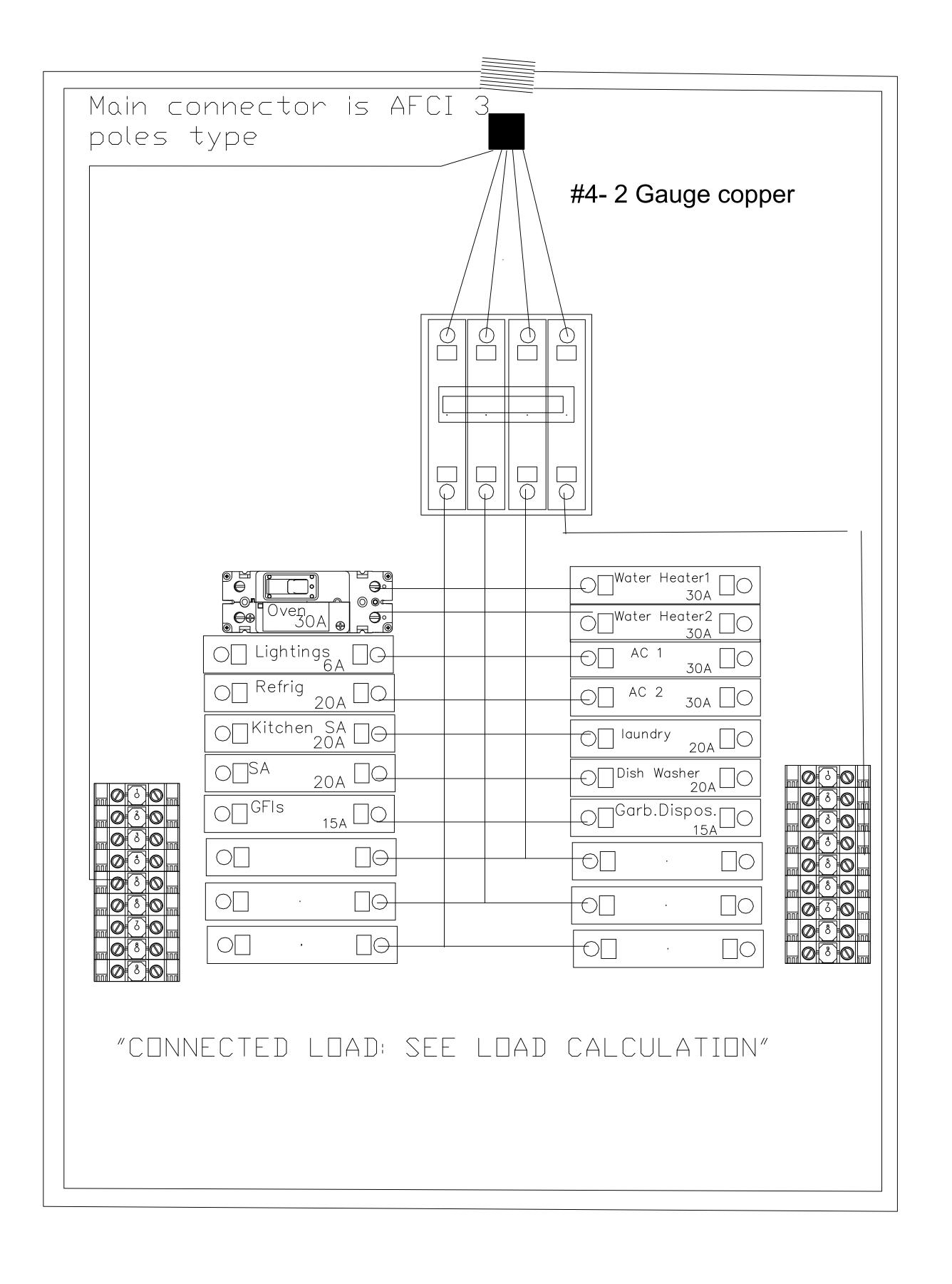
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CEC Section 250.50 Grounding Electrode System and Grounding Electrode Conductor

250.50 Grounding Electrode System. All grounding electrodes as described in 250.52(A)(1)

through (A)(7) that are present at each building or structure served shall be bonded together to

form the grounding electrode system. Where none of these grounding electrodes exist, one or

more of the grounding electrodes specified in 250.52(A)(4) through (A)(8) shall be installed and used.

Exception: Concrete-encased electrodes of existing buildings or structures shall not be

required to be part of the grounding electrode system where the steel reinforcing bars or

rods are not accessible for use without disturbing the concrete. CEC Section 250.104 Bonding of Piping Systems and Exposed Structural Metal.

(A) Metal Water Piping. The metal water piping system shall be bonded as required in (A)(1),

(A)(2), or (A)(3) of this section. The bonding jumper(s) shall be installed in accordance with

250.64 (A), (B), and (E).

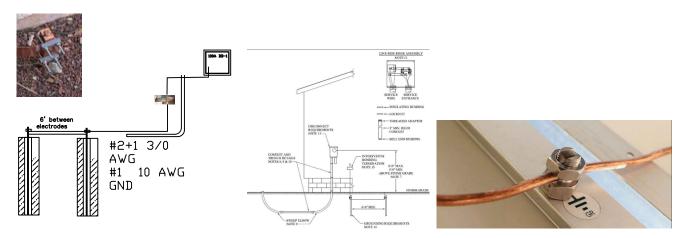
(1) General. Meter water piping system(s) installed in or attached to a building or

structure shall be bonded to the service equipment enclosure, the grounded conductor at

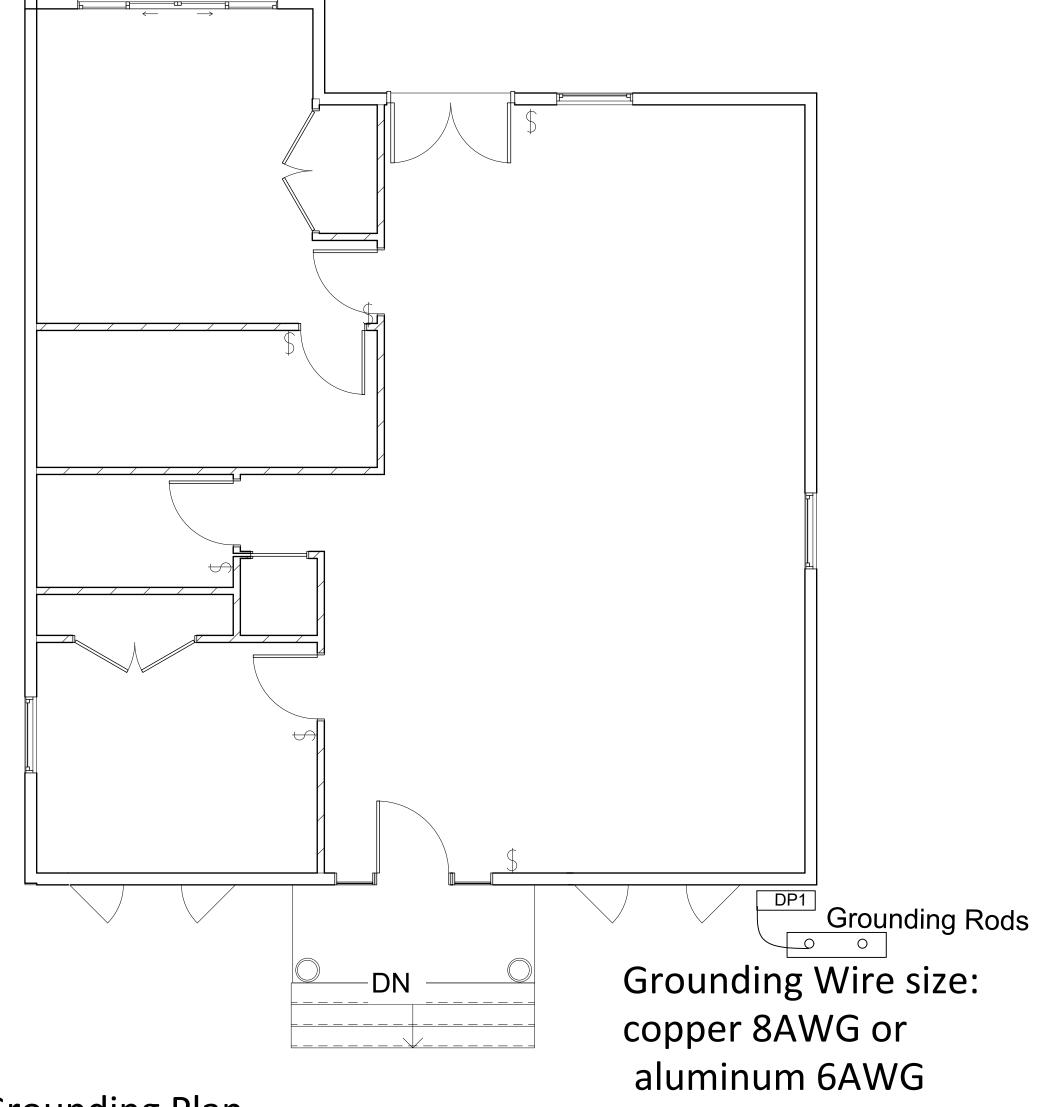
the service, the grounding electrode conductor where of sufficient size, or to the one or

more grounding electrodes used. The bonding jumper(s) shall be sized in accordance

with Table 250.66 except as permitted in 250.104(A)(2) and (A)(3).



2 ground rods must be at least 8 feet buried in the ground with minimum of 6 feet apart. When made of iron or steel, the ground rod must be a minimum 5/8" diameter. Listed stainless steel or nonferrous rods may be 1/2" in diameter. Grounding electrode conductor shall be connected within 5 ft. from the point of entrance to a cold water pipe grounding electrode. (2007 CEC Section 250-30 Item 3)



Grounding Plan

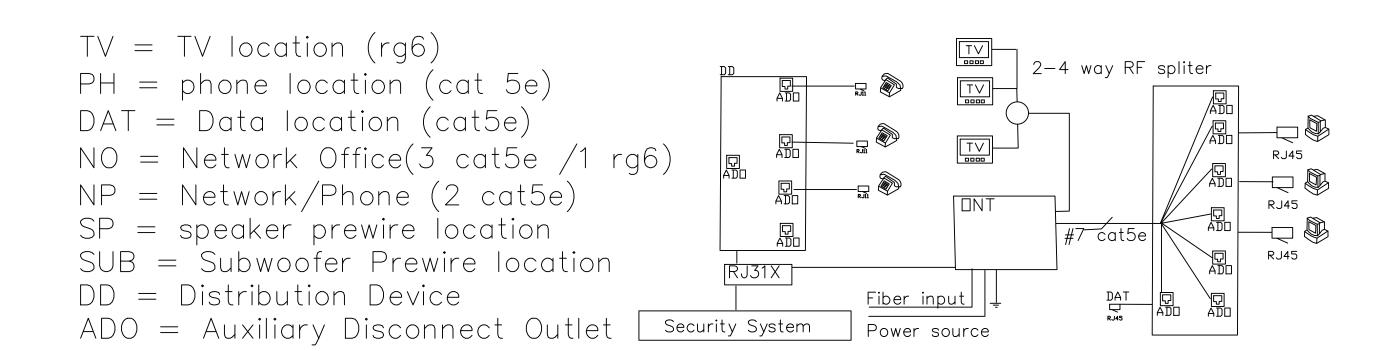
scale: 1/4" = 1'

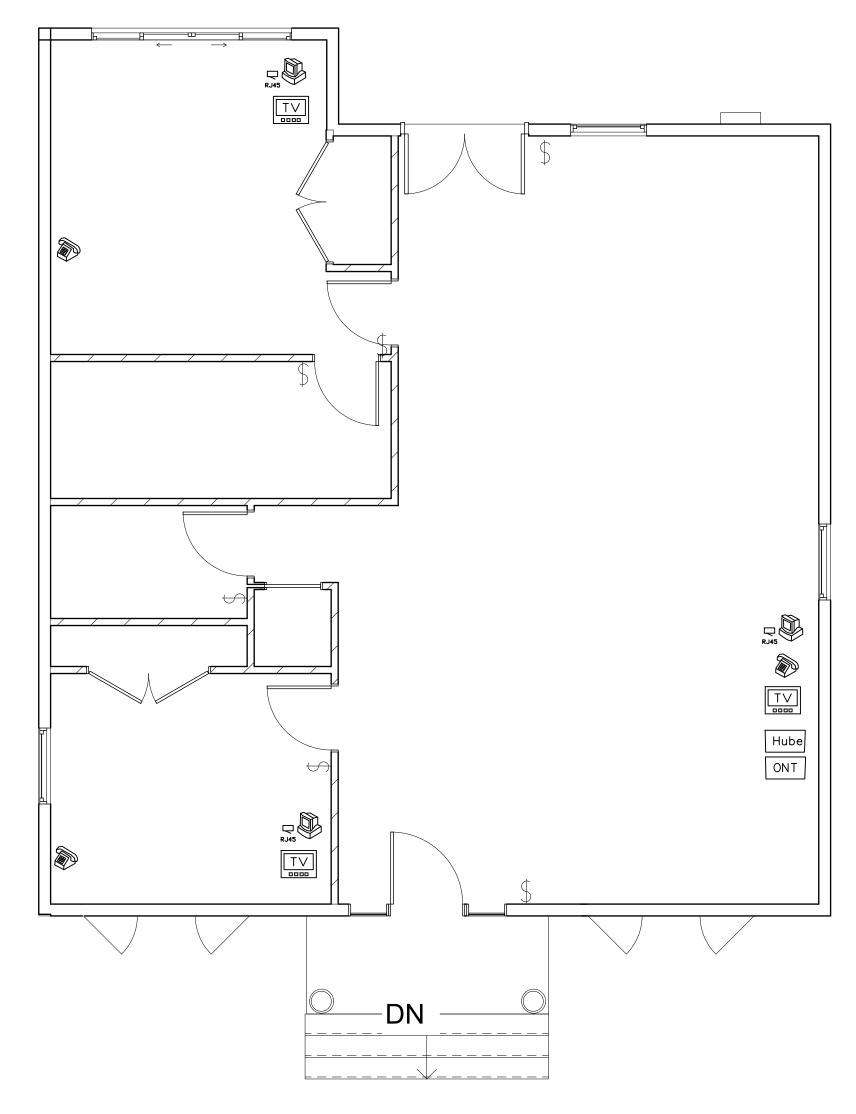


Project Name and Address:

SINGLE FAMILY HOUSE REMODEL

Date:	DRAWING TITLE:	Sheet :	No.	Revision/Issue	Date
	Grounding service	7 OF 9			
Scale: 1/4" = 1' 00"					
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PERMISSION WITH OWNER, PL					





TV and Data Plan

scale: 1/4'' = 1'

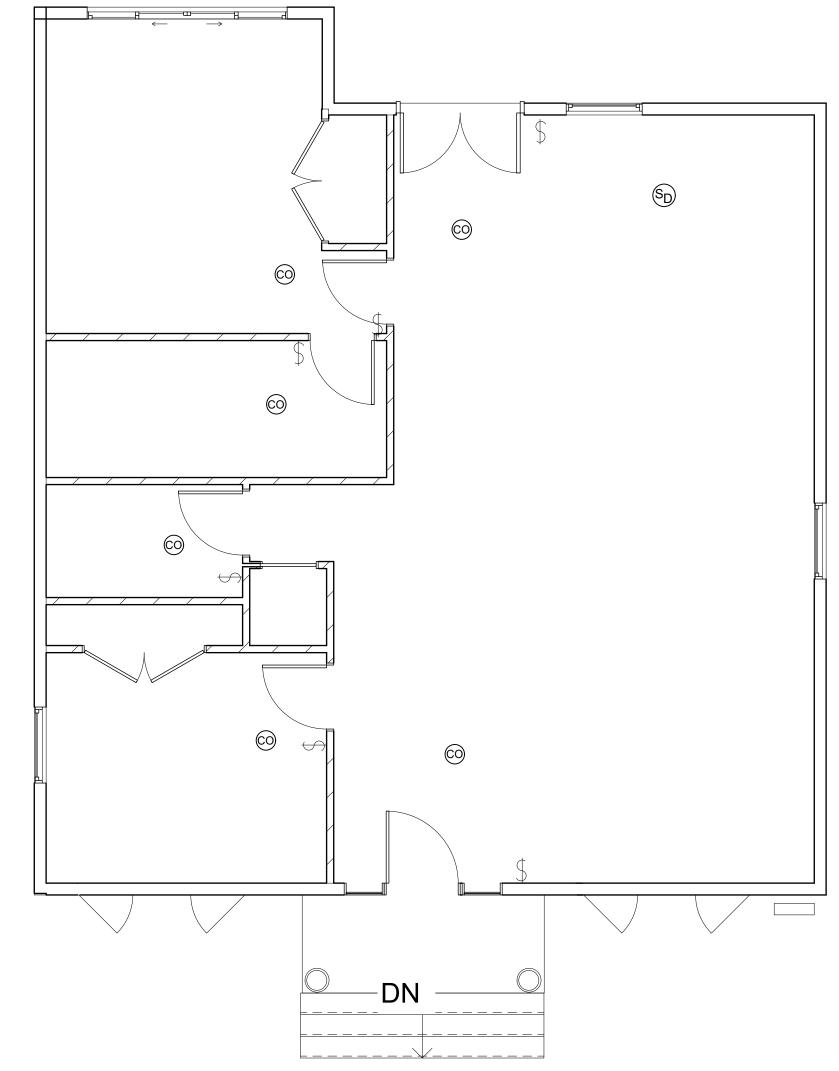




Canada Office 3313Plateau Blvd. Coquitlam BC V3E 3B8 +1 909 939 2585 info@pixelarchltd.com www.pixelarchltd.com

Project Name and Address:

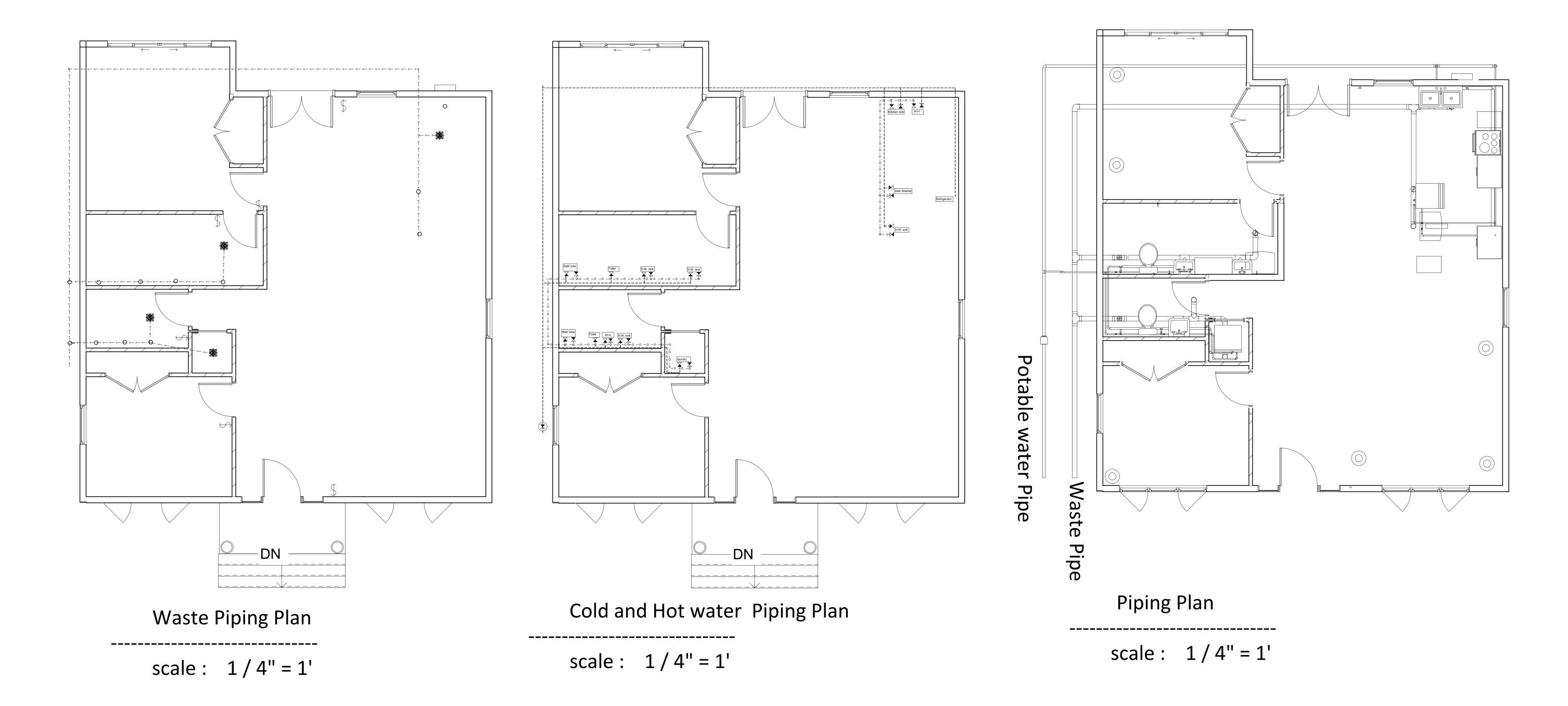
SINGLE FAMILY HOUSE REMODEL



Fire Alarm Plan

scale: 1/4'' = 1'

Date:	DRAWING TITLE:	Sheet :	No.	Revision/Issue	Date
Scale: 1/4" = 1' 00"	DATA and CATV & Fire Alarm	8 OF 9			
	OF SERVICE AND AS SUCH, REMAINS THE PROPERTY OF PIXELARCH ODUCTION IS LIMITED AND CAN BE EXTENDED ONLY BY WRITTEN	Page No. : E06			



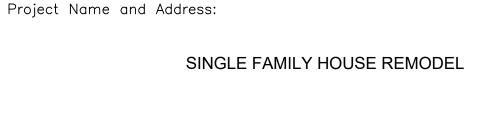
MECH	ANICAL SYMBOLS AND ABBREVIATIONS
-	FLOOR DRAIN
	FUNNEL FLOOR DRAIN
	FLOOR SINK
0- 0- 0-	HOT WATER
	COLD WATER
s— s— s—	Sewer and WASTE
X	Valve
W.H.	Water Heater
H.W.	Hand wash sink

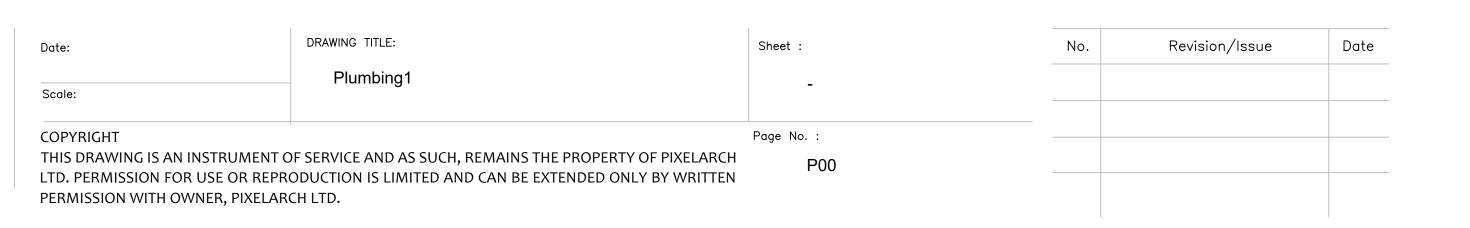
All Outside piping shall be installed under or within 2 feet (610 mm) of a building or structure, or less than 1 foot (305 mm) below the surface of the ground.

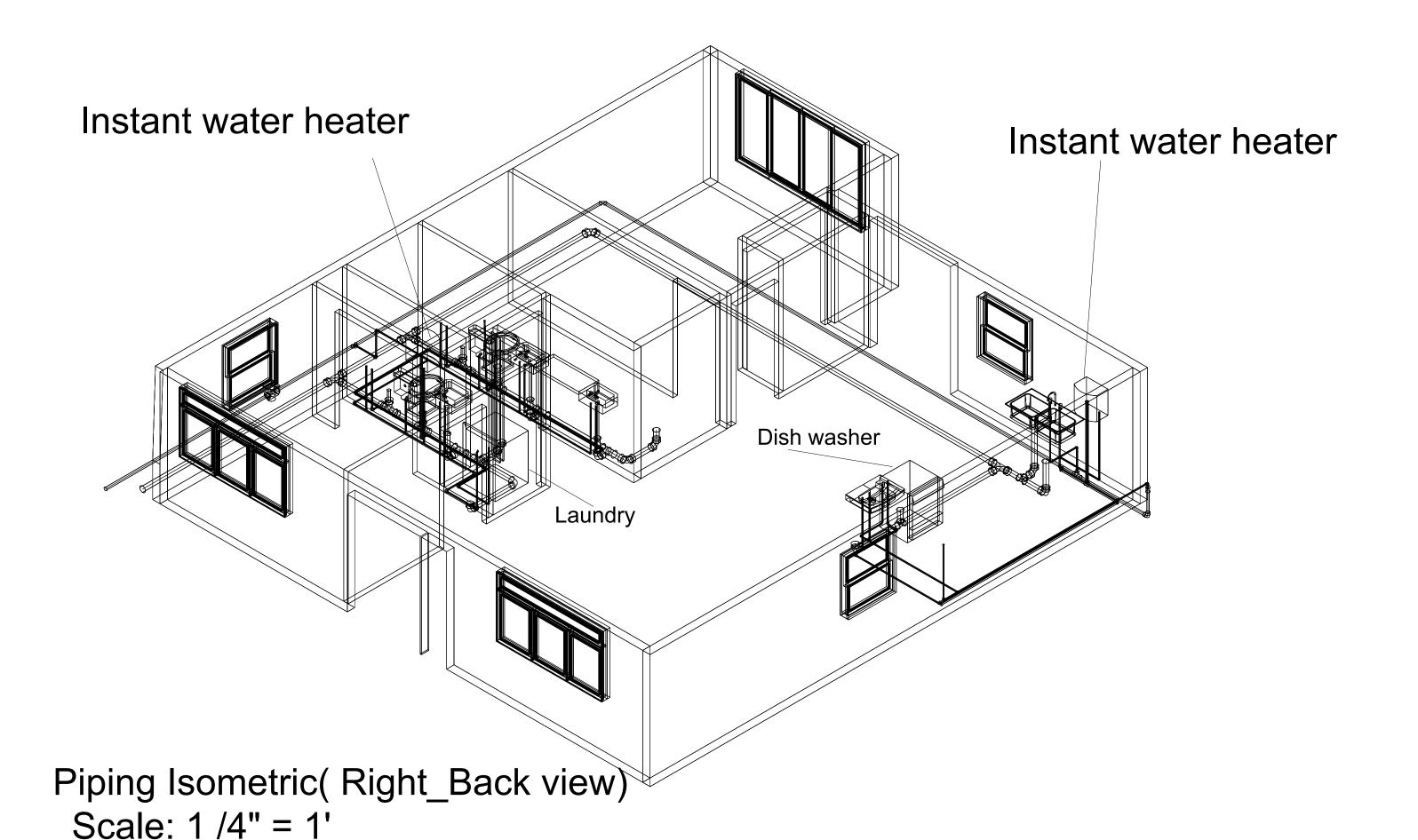
according to CPC.609.11 Pipe Insulation, Insulation of domestic hot water piping shall be in accordance with Section 09.11.1 and Section 609.11.2. of CPC.

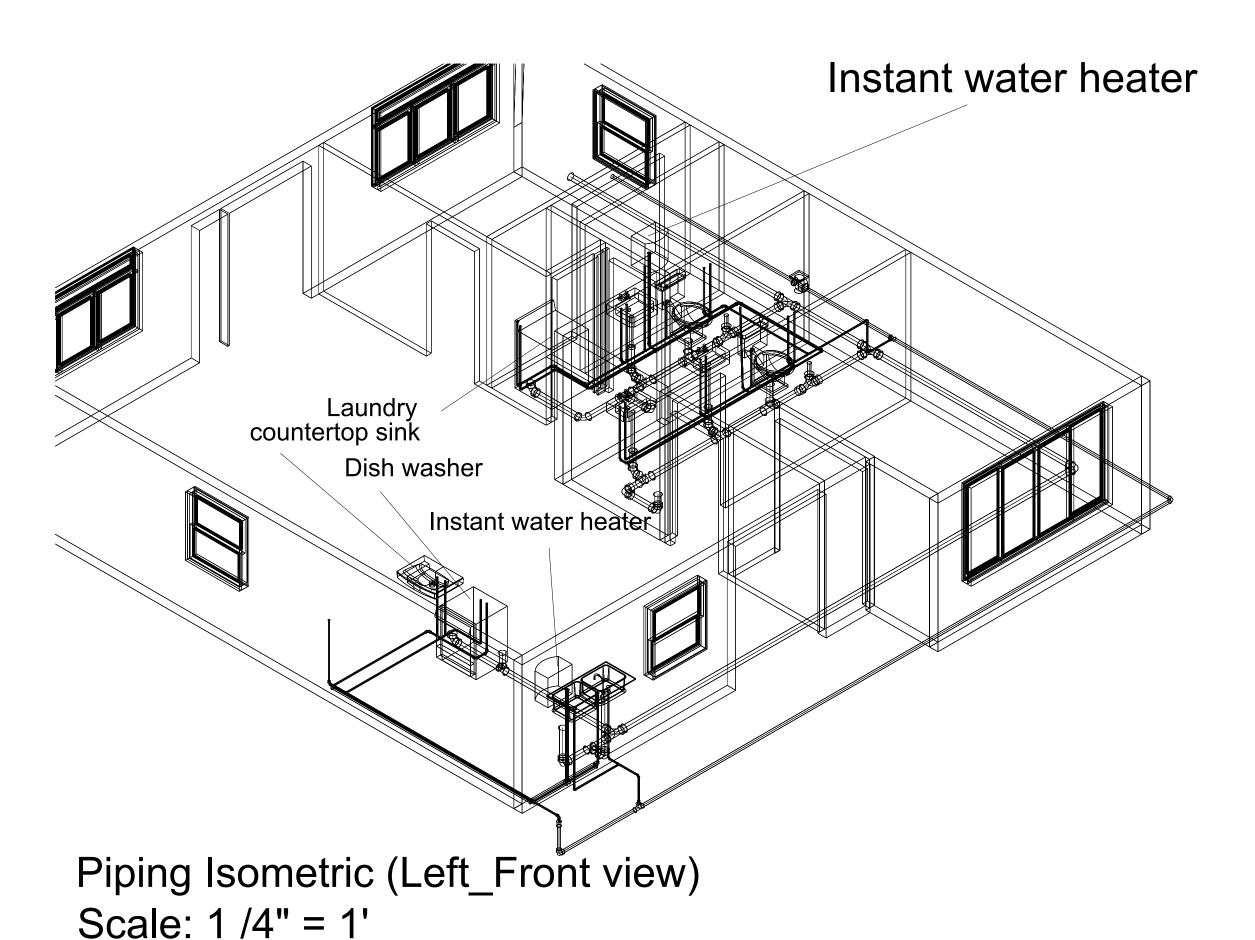
609.11.2 Pipe Insulation Wall Thickness. Hot water pipe insulation shall have a minimum wall thickness of not less than the diameter of the pipe for a pipe up to 2 inches (50 mm) in diameter. Insulation wall thickness shall be not less than 2 inches (51 mm) for a pipe of 2 inches (50 mm) or more in diameter.











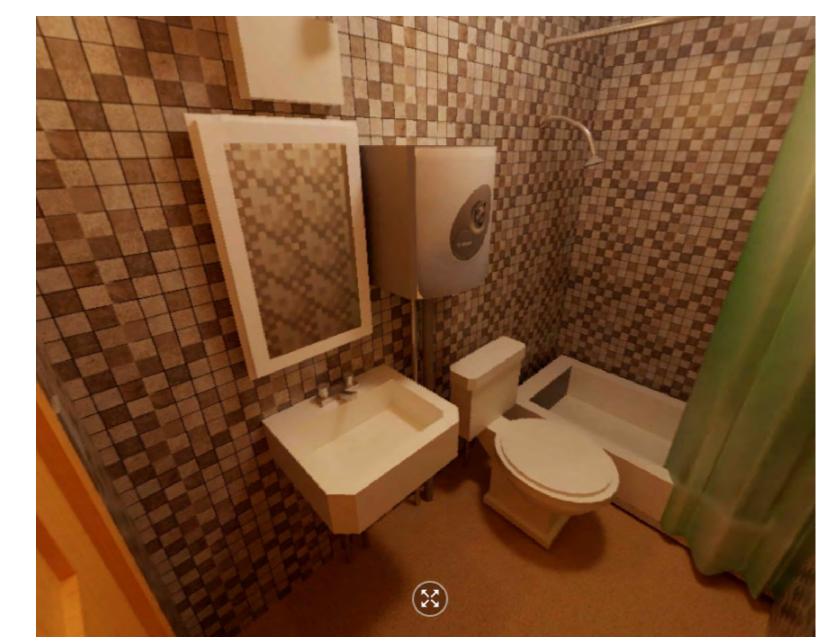
CPC Notes

312.0 Protection of Piping, Materials, and Structures.

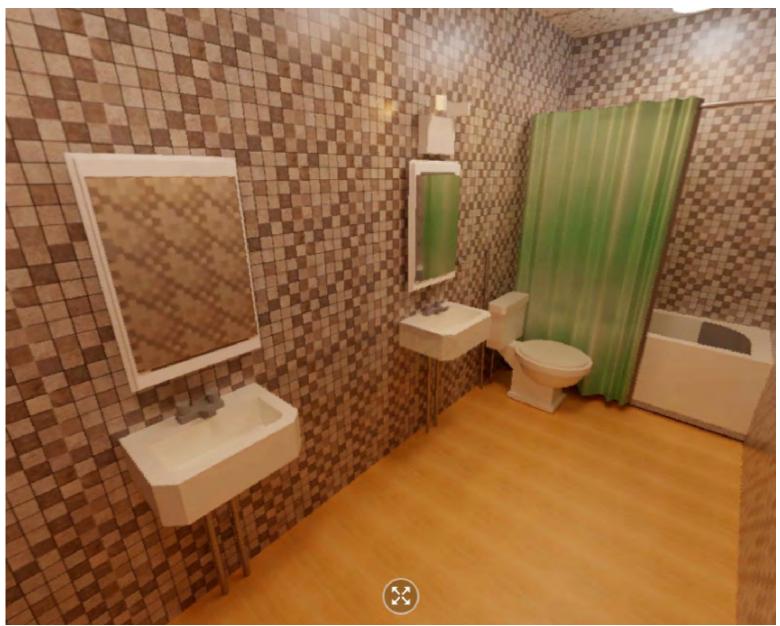
312.1 General. Piping passing under or through walls shall be protected from breakage. Piping passing through or under cinders or other corrosive materials shall be protected from external corrosion in an approved manner. Approved provisions shall be made for expansion of hot water piping. Voids around piping passing through concrete floors on the ground shall be sealed.

312.2 Installation. Piping in connection with a plumbing system shall be so installed that piping or connections will not be subject to undue strains or stresses, and provisions shall be made for expansion, contraction, and structural settlement. No plumbing piping shall be directly embedded in concrete or masonry. No structural member shall be seriously weakened or impaired by cutting, notching, or otherwise, as defined in the California Building Code or California Residential Code. 312.3 Building Sewer and Drainage Piping. No building sewer or other drainage piping or part thereof, constructed of materials other than those approved for use under or within a building, shall be installed under or within 2 feet (610 mm) of a building or structure, or less than 1 foot (305 mm) below the surface of the ground. 312.6 Freezing Protection. No water, soil, or waste pipe shall be installed or permitted outside of a building, in attics or crawl spaces, or in an exterior wall unless, where necessary, adequate provision is made to protect such pipe from freezing.

Bath room view



Master Bath room view







Project Name and Address: SINGLE FAMILY HOUSE REMODEI

te:	DRAWING TITLE:	Sheet :	No.	Revision/Issue	Date
ale:	Cover	-			
PYRIGHT	AF CERVICE AND ACCUCIL REMAINS THE PROPERTY OF RIVELARCH	Page No. :			
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DESIGN CODE:

1. 2016 CBC

DESIGN LOADS:

Floor live load: 40 psf
 Floor dead load: 15 psf
 Roof dead load: 20 psf

4. Roof live load: 20 psf

5. Wind load : 10 psf roof 24psf wall

6. Snow load: 0 pdf 6. Ex Wall DD = 12psf 7. In. Wall DD = 8psf

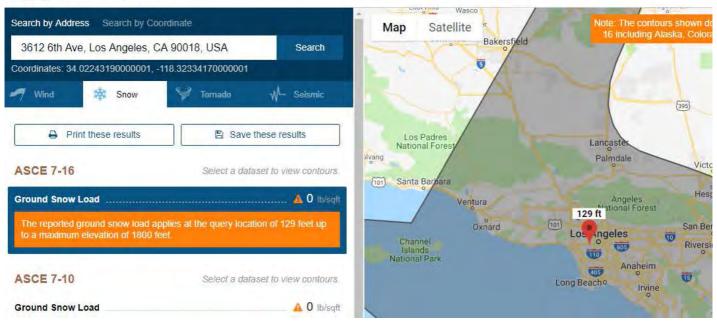
8. Concrete wall 8" – 97psf

9. Concrete 145pcf

ATC Hazards by Location Search by Address Search by Coordinate Castaic ake State Map Satellite Recreation Area 3612 6th Ave, Los Angeles, CA 90018, USA Search Agua Dulce Coordinates: 34.02243190000001, -118.32334170000001 Castaic (14) Snow Seismic Wind Forest Park Val Verde (126) Santa Clarita Print these results Save these results 210 **ASCE 7-16** Select a dataset to view contours. San Fernando 66 mph Simi Valley MRI 10-Year (118) 5 210 71 mph MRI 25-Year 77 mph MRI 50-Year Burbank nousand (170) Calabasas 27 Glenda MRI 100-Year 81 mph Topanga State Park Risk Category I 89 mph Beverly Hills 129 ft 95 mph os An Risk Category II Malibu Santa Monica 102 mph Risk Category III 106 mph Risk Category IV Inglewood

STRUCTURAL CALCULATION

ATC Hazards by Location



Search Information

Address: 3612 6th Ave, Los Angeles, CA 90018, USA

Coordinates: 34.02243190000001, -118.32334170000001

Timestamp: 2018-12-25T15:51:08.958Z

Hazard Type: Seismic

Reference Document: ASCE7-16

Risk Category:

Site Class: D-default

Report Title: Not specified

Map Results



Text Results

Basic Parameters

Name	Value	Description
SS	1.936	MCE _R ground motion (period=0.2s)
S ₁	0.684	MCE _R ground motion (period=1.0s)
S _{MS}	2.323	Site-modified spectral acceleration value
S _{M1}	* null	Site-modified spectral acceleration value
S _{DS}	1.549	Numeric seismic design value at 0.2s SA
S _{D1}	* null	Numeric seismic design value at 1.0s SA

^{*} See Section 11.4.8

Additional Information

Name	Value	Description
SDC	* null	Seismic design category
Fa	1.2	Site amplification factor at 0.2s
F _v	* null	Site amplification factor at 1.0s
PGA	0.827	MCE _G peak ground acceleration
F _{PGA}	1.2	Site amplification factor at PGA

12/25/2018		ATC Hazards by Location	
PGA _M	0.993	Site modified peak ground acceleration	
TL	8	Long-period transition period (s)	
SsRT	1.936	Probabilistic risk-targeted ground motion (0.2s)	
SsUH	2.14	Factored uniform-hazard spectral acceleration (2% probability of exceedance in 50 years)	
SsD	2.376	Factored deterministic acceleration value (0.2s)	
S1RT	0.684	Probabilistic risk-targeted ground motion (1.0s)	
S1UH	0.759	Factored uniform-hazard spectral acceleration (2% probability of exceedance in 50 years)	
S1D	0.795	Factored deterministic acceleration value (1.0s)	
PGAd	0.958	Factored deterministic acceleration value (PGA)	

^{*} See Section 11.4.8

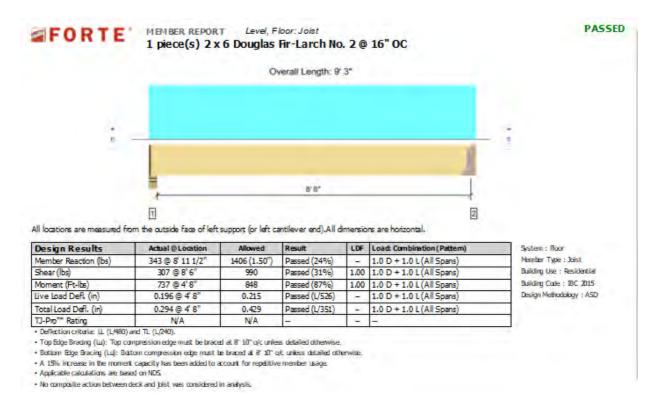
The results indicated here DO NOT reflect any state or local amendments to the values or any delineation lines made during the building code adoption process. Users should confirm any output obtained from this tool with the local Authority Having Jurisdiction before proceeding with design.

Disclaimer

Hazard loads are provided by the United States Geological Survey Seismic Design Web Services.

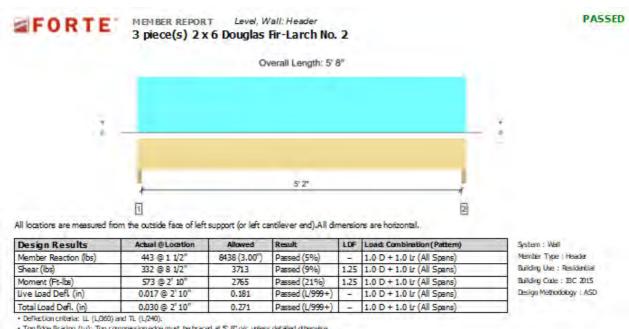
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DETERMINATION OF FLOOR JOIST



We use same as existing but not less as 2x6 DF#2

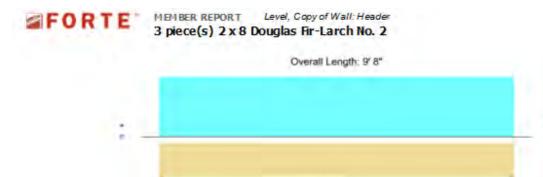
DETERMINATION OF HEADER ABOVE REAR FRENCH DOOR



- Top Edge Bracing (tu): Top compression edge must be braced at 5° 8° o/c unless detailed differvise.
- + Bottom Bige Bracing (Lu): Bdtom compression edge must be braced at 5° 8° o/c unless detailed otherwise.
- Applicable calculations are based on NDS.

DETERMINATION ON HEADER ABOVE REAR SLIDING DOOR

PASSED



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Actual @ Location LDF Load: Combination (Pattern) Design Results Allowed 1055 @ 1 1/2 8438 (3.00) Passed (13%) 1.0 D + 1.0 Lr (All Spans) Member Reaction (lbs) Shear (lbs) 869 @ 10 1/4 4894 Passed (18%) 1.0 D + 1.0 lr (All Spans) 1.25 1.0 D + 1.0 tr (All Spans) Moment (Ft-lbs) 2419 @ 4' 10' 4435 Passed (55%) Live Load Defl. (in) 0.070 @ 4' 10' 0.314 Passed (L/999+) 1.0 D + 1.0 Lr (All Spans) Total Load Defl. (in) 0.169 @ 4' 10" 0.313 Passed (L/669) 1.0 D + 1.0 Lr (All Spans)

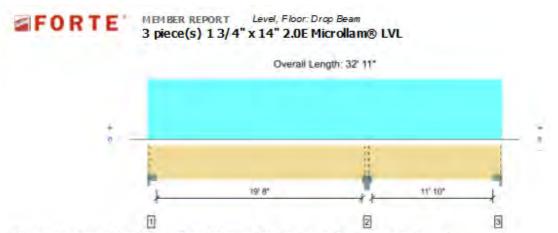
System : Wall Member Type : Header Building Use : Residential Building Cade : IBC 2015 Design Methodology : ASD

PASSED

2

- Deflection criteria: LL (L/350) and TL (5/16").
- Top Edge Brading (Lu): Top compression edge must be braced at 9° 8" o/c unless detailed ditherwise.
- Bottom Edge Bracing (Lul): Buttom compression edge must be braced at 9 8" o/c unless datailed otherwise.
- · Applicable calculations are based on NDS

DETERMINATION OF BEAM



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	13472 @ 20' 4 1/2"	23625 (6.00")	Passed (57%)	-	1.0 D + 1.0 tr (All Spans)
Shear (lbs)	6691 @ 18' 11 1/2"	17456	Passed (38%)	1.25	1.0 D + 1.0 tr (All Spans)
Moment (Ft-lbs)	-24347 @ 20' 4 1/2"	45484	Passed (54%)	1.25	1.0 D + 1.0 Ir (All Spans)
Live Load Defl. (in)	0.266 @ 9' 5 7/8"	0.668	Passed (L/906)	-	1.0 D + 1.0 Ir (Alt Spans)
Total Load Defl. (in)	0.596 @ 9' 4 15/16"	1.002	Passed (L/403)		1.0 D + 1.0 Ir (Alt Spans)

System: Floor Member Type: Drop Beam Building Use: Residential Building Code: IBC 2015 Design Methodology: ASD

- Deflection criteria: U. (L/360) and TL (L/240).
- Top Edge Bracing (Lu): Top compression edge must be braced at 15' 8" o/c unless detailed otherwise.
- Boltom Edge Bracing (Lu): Bottom compression edge must be braced at 13' 3" oc unless detailed otherwise.

DETERMINATION SIZE OF FOUNDATION

TYPE OF LOAD	TRIBUTARY WIDTH		LOAD		TOTAL
1ST DD=	16,50	х	20	II	330,00
1ST LL=	16,50	X	40	Ш	660,00
ROOF DD=	17,50	X	20	II	350,00
RL=	17,50	х	20	=	350,00
W=	17,50	х	10	=	175,00
S=	17,50	х	0	=	0,00
W wall=	16	х	12	=	192,00
FOUNDATION WALL	1,33	х	145	=	129,21
FOOTING	1.5X2	Χ	145	=	435,00

TOTAL DD 1436,21 LBS

F=	1.2XDD+1.6XLL+0.5LR	2954,45	LBS
F=	1.2XDD+1.6XLR+1XLL	2943,45	LBS
F=	1.2XDD+1.6XLR+0.8XW	2423,45	LBS
F=	1.2XDD+1.6XW+1XL+0.5LR	2838,45	LBS

Fmax/1500

= 1,97 sf

We use 2'-0" x 16"D footing

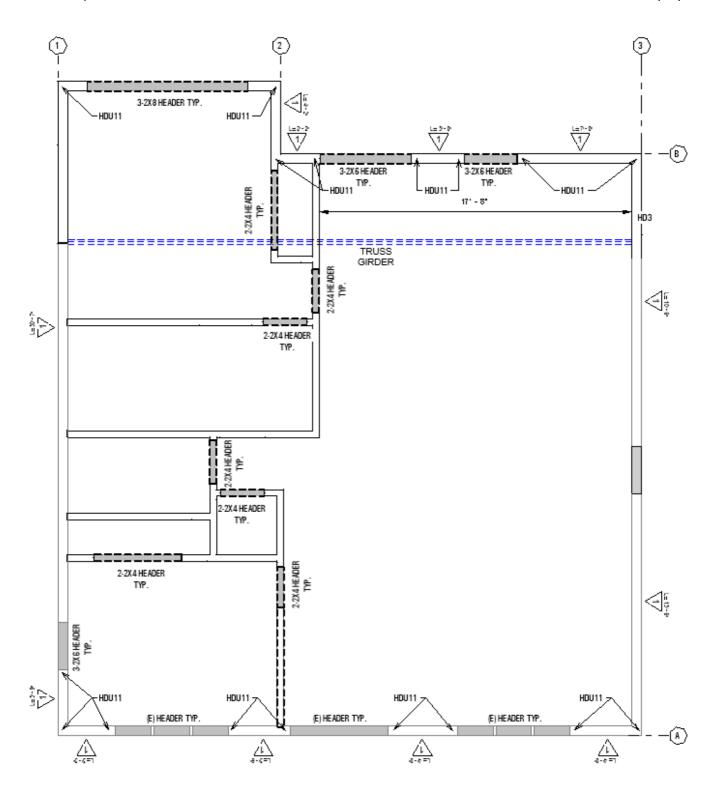
DETERMINATION SIZE OF FOUNDATION UNDER MIDDLE SUPPORT

Reaction - 16365 LBS

16365/1500=10.93 <3.5X3.5=12.25

We use footing pad 42"x42"x24".

LATERAL ANALYSIS



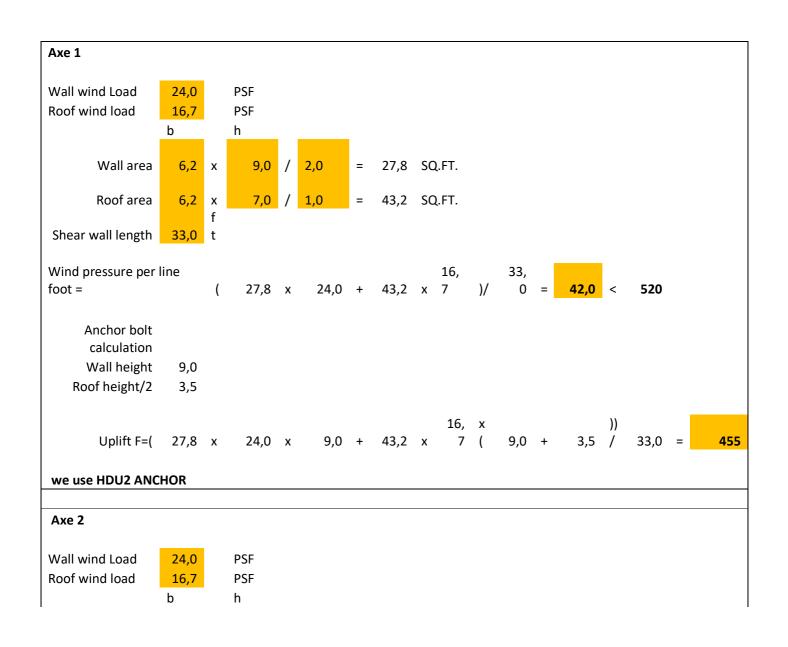
Wind loads analysis

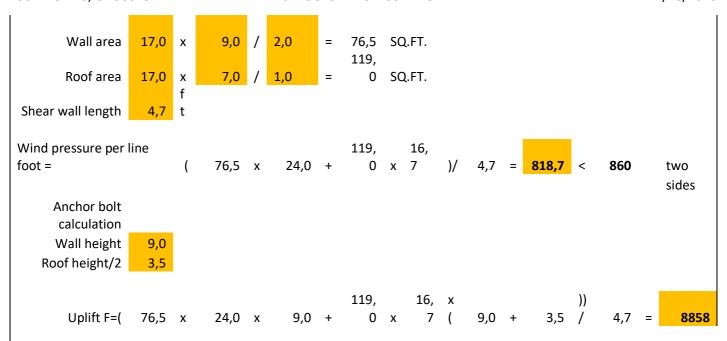
Type of plywood Table 4.3A	Anchor capacity
----------------------------	-----------------

STRUCTURAL CALCULATION

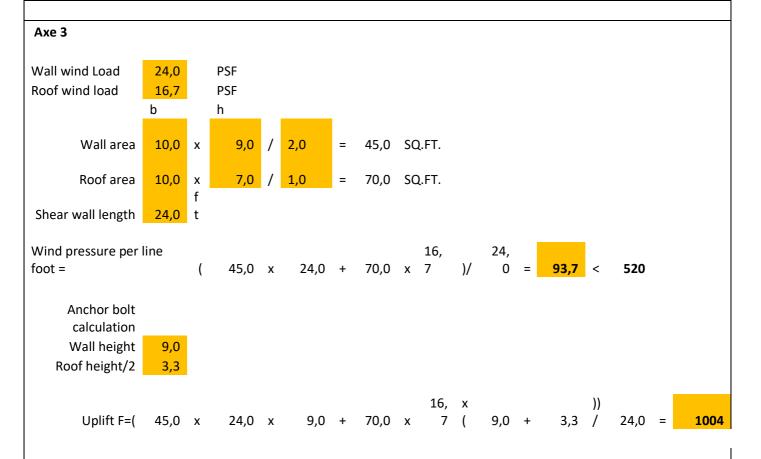
REV 1 12/28/2018

				HDU2	HDU4	HDU5	HDU8	HDU11	HDU14
				_	_			_	
	6,0	4,0	3,0	SDS2.5	SDS2.5	SDS2.5	SDS2.5	SDS2.5	SDS2.5
15/32 8d 1-3/8	730,	1065,	1370,						10770,
13/32 ou 1-3/6	0	0	0	3075,0	4565,0	5645,0	6765,0	9535,0	0
	365,							11175,	14390,
	0	532,5	685,0				6970,0	0	0
15/32 10d 1-1/2	870,	1290,	1680,						14445,
15/52 100 1-1/2	0	0	0				7870,0		0
	435,								
	0	645,0	840,0						
	950,	1430,	1860,						
10/22 104 1 1/2	0	0	0						
19/32 10d 1-1/2	475,								
	0	715,0	930,0						

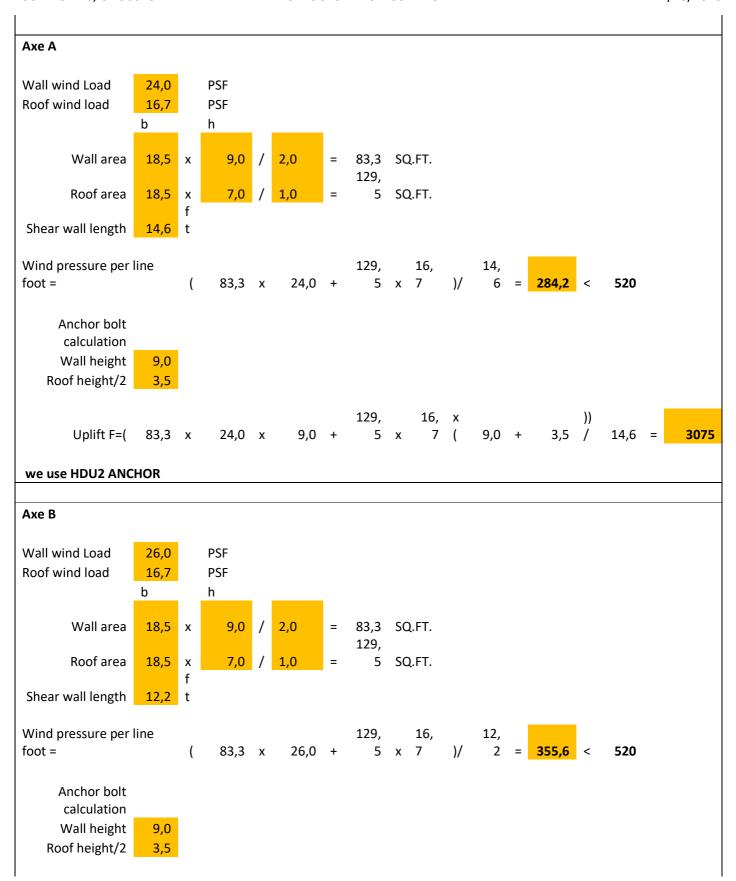




we use HDU11 ANCHOR



we use HDU5 ANCHOR



REV 1 12/28/2018

	STRIBUTION	<u>l</u>							
ROOF									
	ROOF WE		20	PSF					
		N WEIGHT	5	PSF					
	W TOTAL		25	PSF	WT				
2ND FLO									
	FLOOR W		0	PSF					
		N WEIGHT	5	PSF					
	W TOTAL		25	PSF					
	HEIGHT		0	СТ					
	:		9	FT					
2046 606	/ 2245 126	SEO 4540 ASSE 7	40.65						
2016 CBC	: / 2015 IBC,	SEC. 1613; ASCE 7	10, St	.C. 12.8					
	V = 0.7 v	(C a v \\\) v a		р.	6 F		1	CDC.	Б
		(C s x W) x ρ		R:	6,5	l:	1	SDC: Sd1:	D 0.727
	Cs = Sds/	(R/T)		S1:	0,684	Sds: Occ.	2	Site	0,727
	Cs=	0,2979				Cat:	П	Class:	D
		0,2010						0.000.	_
Chook C	onstraints								
Crieck C	ภางแลก เง								
Crieck Co		0.044 * I * S _{DS}							
Crieck Co	Cs min = 0	0.044 * I * Sps							
Cs min	Cs min = 0	0.044 * I * S _{DS} : S _{D1} / T (R / I)							
	Cs min = 0								
Cs min	Cs min = 0 Cs max =								
Cs min	Cs min = 0 Cs max = 0,085								
Cs min	Cs min = 0 Cs max = 0,085	$S_{D1} / T (R / I)$ $S_{D1} = 2/3 * S_{M1}$ ASCE 7-02 Eq. 9.4.1.2.5-	2						
Cs min	Cs min = 0 Cs max = 0,085	S _{D1} / T (R / I) S _{D1} = 2/3 * S _{M1} ASCE 7-02 Eq. 9.4.1.2.5- S _{M1} = Fv *	2						
Cs min	Cs min = 0 Cs max = 0,085	$S_{D1} / T (R / I)$ $S_{D1} = 2/3 * S_{M1}$ ASCE 7-02 Eq. 9.4.1.2.5-	2	S ₁ ^a =	0,684	S _{M1} =	1,09		
Cs min	Cs min = 0 Cs max = 0,085	S _{D1} / T (R / I) S _{D1} = 2/3 * S _{M1} ASCE 7-02 Eq. 9.4.1.2.5- S _{M1} = Fv *		S ₁ ^a = Fv ^a =	0,684 1,2	S _{M1} =	1,09		
Cs min	Cs min = (Cs max = 0,085 For S _{D1} :	$S_{D1} / T (R / I)$ $S_{D1} = 2/3 * S_{M1}$ ASCE 7-02 Eq. 9.4.1.2.5- $S_{M1} = Fv *$ S_{1} ASCE 7-02 Eq. 9.4.1.2.4-				S _{M1} =	1,09		
Cs min	Cs min = 0 Cs max = 0,085	$S_{D1} / T (R / I)$ $S_{D1} = 2/3 * S_{M1}$ ASCE 7-02 Eq. 9.4.1.2.5- $S_{M1} = Fv *$ S_{1}				S _{M1} =	1,09		
Cs min	Cs min = 0 Cs max = 0,085 For S _{D1} :	S _{D1} / T (R / I) S _{D1} = 2/3 * S _{M1} ASCE 7-02 Eq. 9.4.1.2.5- S _{M1} = Fv * S ₁ ASCE 7-02 Eq. 9.4.1.2.4- 0,727		Fv ^a =	1,2		1,09		
Cs min	Cs min = (Cs max = 0,085 For S _{D1} :	$S_{D1} / T (R / I)$ $S_{D1} = 2/3 * S_{M1}$ $ASCE 7-02 Eq. 9.4.1.2.5-S_{M1} = Fv * S_1$ ASCE 7-02 Eq. 9.4.1.2.4- 0,727 T = Cu * Ta		Fv ^a =	1,2	S _{M1} =	1,09		
Cs min	Cs min = 0 Cs max = 0,085 For S _{D1} :	S _{D1} / T (R / I) S _{D1} = 2/3 * S _{M1} ASCE 7-02 Eq. 9.4.1.2.5- S _{M1} = Fv * S ₁ ASCE 7-02 Eq. 9.4.1.2.4- 0,727		Fv ^a =	1,2		1,09		

REV 1 12/28/2018

Ta = 0,104 T = 0,145

Cs max

Cs FINAL = 0,2979

0,9605

 $V = 0.7 x (Cs x W) x \rho =$

0,27108

V= 13,5538

		W TOTA	Н	WT*H			
wt ht wt*ht % F		L	TOTAL	Т	%	F	V TOTAL
ROOF		25	9	225	1,00	13,55	13,55

HOLD DOWN CAPACITIES SHEAR WALL CAPACITIES

HDU

HDU2 HDU4 2307

HDU5 HDU8

3425

4254

11

HDU14 5904 7152 10835 lbs

SHEAR WALL DESIGN

TYPE 2	TYPE 3	TYPE 4	TYPE 5	
280	430	550	730	lbs/ft
560*	860*	1100	1460	

SHEAR WALL DESIGN

Shear Line	Level										
										T NET	
		TRIBUTAR		F			WAL		DL/FL	WALL	
	LENGTH	Y AREA	F FLR	ADD	F TOTAL	V/FT	L	T/C	R	DL	HDU
Line A	FT	SQ.FT	LBS	LBS	LBS	PLF	TYPE	LBS			
1ST	14,50	545,00	7386,79		7386,79	509,43	4	4584,91	80,00	4005	HDU5
Line B											
1ST	12,17	591,36	8015,15		8015,15	658,78	5	5929,01	80,00	5442	HDU8
Line 1											
1ST	33,00	569,00	7712,08		7712,08	233,70	2	2103,30	80,00	783	HDU2
Line 2											
1ST	4,67	26,00	352,40		352,40	75,51	1	679,62	80,00	493	HDU2
Line 3											
1ST	24,00	543,00	7359,69		7359,69	306,65	3	2759,88	80,00	1800	HDU2